

THE APPLICATION OF THE RECEIVING WATER QUANTITY
MODEL TO THE CONSERVATION AREAS OF SOUTH FLORIDA

BY

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ABSTRACT

The conservation areas of South Florida Everglades are major water storage areas to provide a water supply for the Everglades National Park and Lower East Coast (LEC). Due to the increasing water demands of the area, additional backpumping of the surplus runoff from the LEC area into the conservation areas is being considered as one of several water supply schemes. The Receiving Water Quantity (EPA, 1971) model was adapted and modified for the Conservation Areas to investigate the hydraulic impact of additional inflow under various backpumping cases. The various modifications to the original EPA model are related to Manning's roughness coefficient, depth of flow, width of hypothetical channels through marsh areas, rainfall input, seepage rate, and the use of the Monte Carlo technique for nodal area computations. Comparison of values simulated by this modified model with the recorded values in Conservation Areas 1 and 2A indicated that (a) the model can simulate satisfactorily the hydraulic regime of the Conservation Areas system during wet seasons, and (b) the model can be a useful tool to study the impact of additional inflow resulting from the backpumping as a part of water use planning and management tasks.

NOTATIONS OF KEY TERMS

V	Velocity (ft/sec),
X	Distance along the channel (ft),
H	Water surface elevation above mean sea level,
g	Gravitational acceleration (32.16 ft/sec^2)
F_f	Acceleration due to fluid resistance (ft/sec^2),
F_w	Acceleration due to wind stress (ft/sec^2),
n	Manning roughness factor
R	Hydraulic radius
t	Time
A_j	Water surface area at node j,
Q_i	Flow through incoming link i,
Q_j	Outflow at node j,
B	Width of canal or hypothetical channel
Δt	time interval,
α	proportional constant
ΔX	channel unit length
h	depth of water
\sqrt{gh}	celerity of the wave

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INTRODUCTION

With the typical seasonal nature of the rainfall pattern coupled with the relatively flat topography of south Florida, the distribution of rainfall inputs in various interconnected parts of a water system becomes a significant piece of information in various water management tasks of planning, regulation and operations. Depending upon the complexities associated with the water system under investigation, the methodology of hydraulic and hydrologic computations varies accordingly. As a part of planning efforts toward the development of the Water Use Plan for the Lower East Coast of South Florida, the South Florida Water Management District has considered several water management schemes to increase the water capabilities of the region to respond to the expected future water demands. One such alternative is the backpumping scheme in which excess surface runoff is to be pumped westward into storage areas instead of allowing that excess runoff to flow eastward to the ocean through the existing canal systems. The storage areas considered in such backpumping alternatives are known as conservation areas of the Florida Everglades and they are covered with sawgrass, red bays, willows, myrtles and various slough aquatics. Currently, these areas function as a water supply storage area for urban, municipal and agricultural uses (including water conveyance to the Everglades National Park). Their indirect functions also include flood control, groundwater recharge, prevention of salt water intrusion and fish, wildlife preservation. Considering these valuable functions performed by the conservation areas, the effects of the increased water quantities as envisioned in backpumping alternatives need to be examined. Furthermore, such assessment should be completed in the present tense for the conditions that are likely to occur in the year 1990 or 2000 or 2020. This particular

requirement demands very definitely a mathematical modeling procedure which can adequately simulate the real world complexities of the conservation areas and then estimate consequences of the future water management schemes. To fulfill such a need, the receiving water quantity model of the EPA's storm water management model (known as SWMM model) is explored to estimate the hydraulic distribution of inputs in the conservation areas.

SPECIFIC OBJECTIVES

Basic objectives of applying the receiving water quantity model are:

1. To modify the receiving water quantity model to describe the flow pattern in the interconnected marsh-canal system of the conservation areas,
2. To simulate the discharges and stages at various locations in the conservation areas,
3. To calibrate the model to the extent possible by comparing simulated stages with the historical stages.
4. To perform sensitivity analysis or a trial-error procedure to select optimum parameters and
5. To superimpose backpumping inputs to estimate change in hydraulic regime of the areas under backpumping conditions.

DESCRIPTION OF THE MODEL

The receiving water body of each of three conservation areas is a continuous system even though the flow through their heavily vegetated area is extremely slow as compared to the flow in canals. The water system in three conservation areas is first represented by a network system of nodes and links as shown in Figures 1, 2 and 3. The shape of the grid system is flexible and they can be either orthogonal or triangular or irregular although an acute triangular shape is recommended.

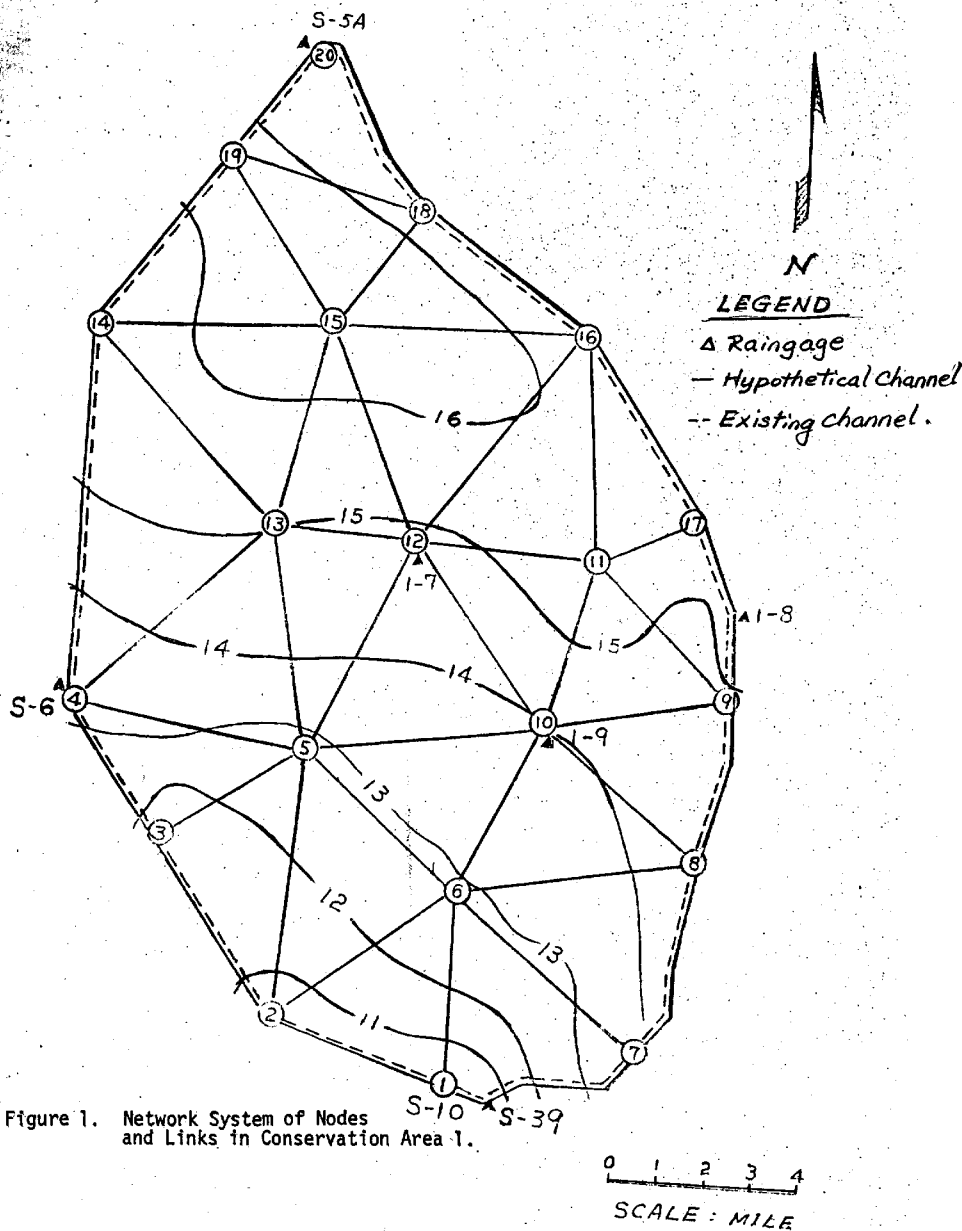


Figure 1. Network System of Nodes and Links in Conservation Area 1.

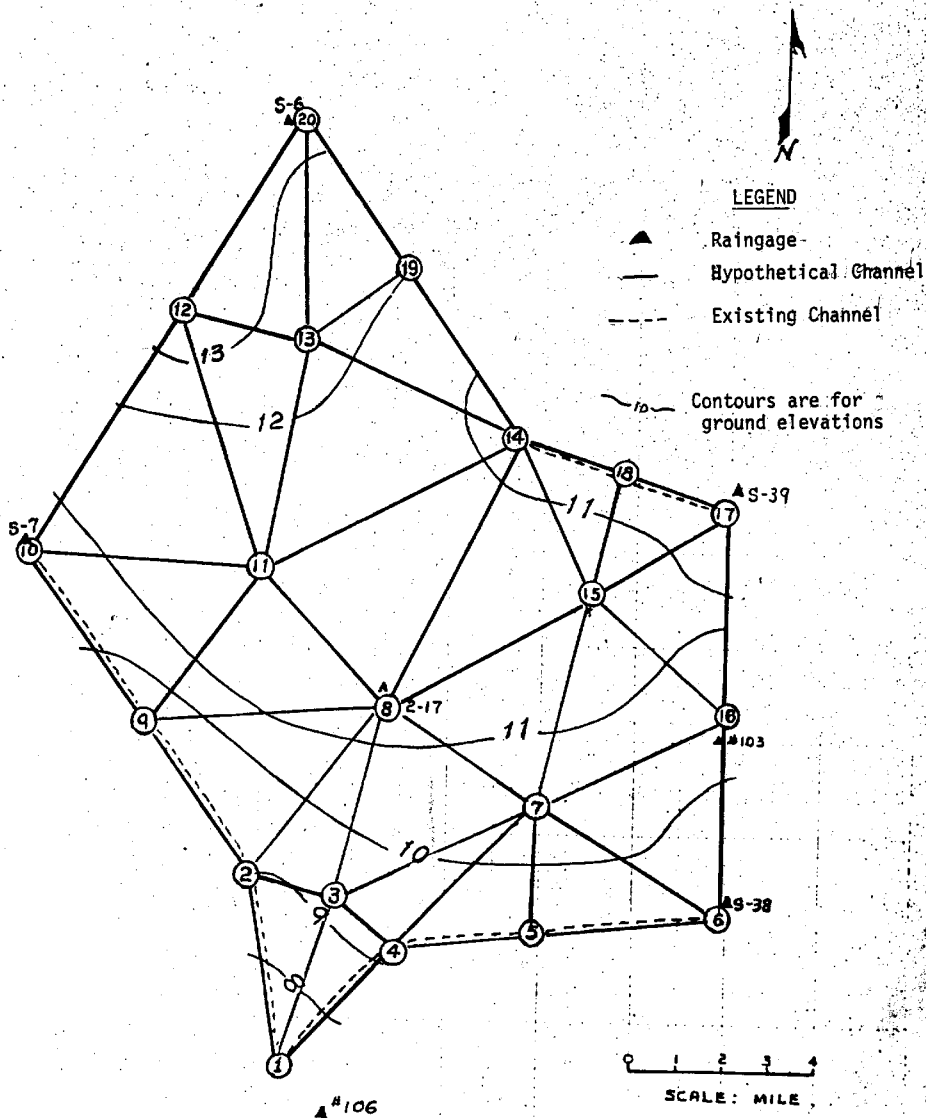


Figure 2. Network System of Nodes and Links in Conservation Area 2A

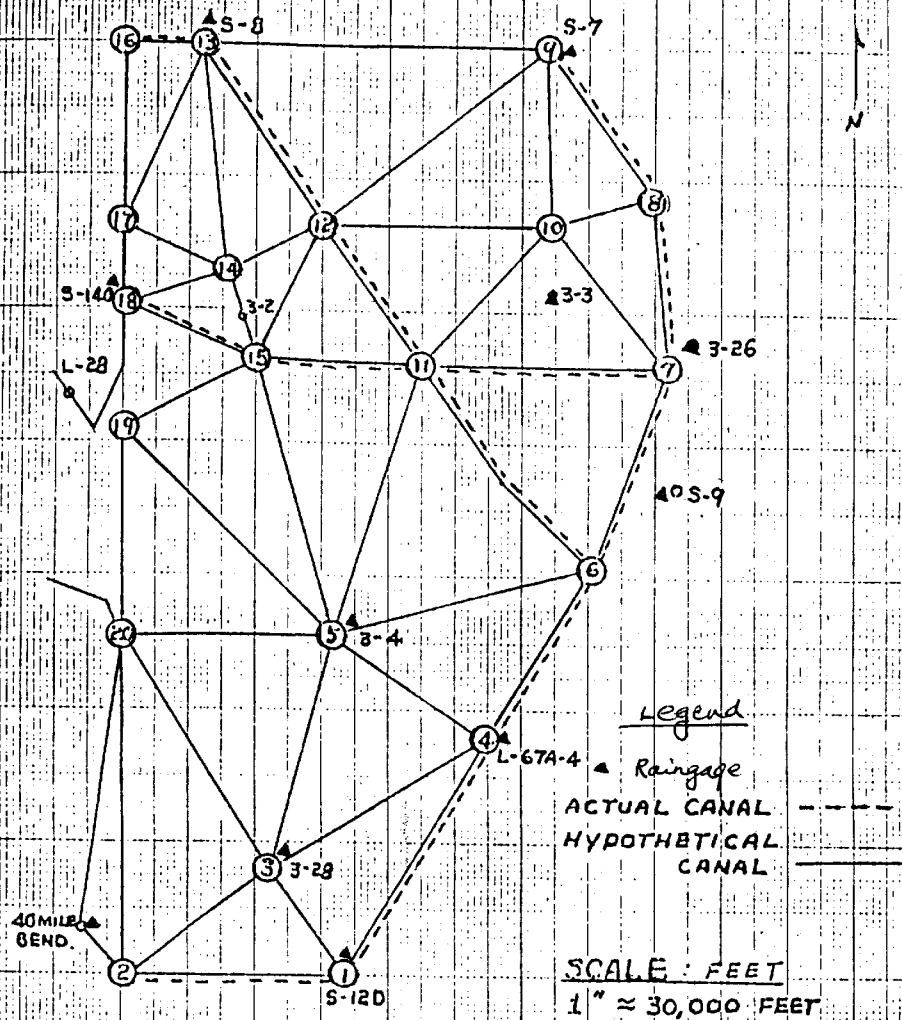


Figure 3. LINK-NODE REPRESENTATION OF CONSERVATION AREA 3A

Conservation Areas 1 and 2A represents two different marsh and canal flow systems. Both areas are surrounded by a dike (levee); however, the deep canal exists all around Area 1 but not Area 2A. The flow movement is generally southward. Sawgrass is the dominant vegetation, occurring as extensive marshes interspersed with tree islands on slightly higher ground elevations. Ground elevations range from 16.5 ft. msl in the northern portion to 11.0 ft. msl at the southern end of Area 1 and from 13.2 ft. msl down to 7.5 ft msl in Area 2A. The network system of area 1 contains 20 nodes and 57 channels, and 51 channels for Area 2A as shown in Figs. 1 and 2. Total area in Area 1 and 2A are approximately 215 and 175 square miles, respectively.

Rainfall stations in Area 1 are S-5A, Gages 1-8, 1-7, 1-9, S-6, and S-39 with only 1-7, and 199 in the interior marsh area. S-10 structures are used as inflow points to Area 2A. Rainfall data for Area 2A are based on the gages at S-6, S-39, S-7, 2-17, FCD #103 and #106 etc. Gage 2-17 is the only gage located inside the interior marsh area, a stage recorder is available on this gage. This gage has been used as an indicator gage to operate S-11 structures. The flow from S-10 travels gradually southward and moves through a vast marsh area before reaching S-11. Therefore, the flow characteristics in Area 2A are quite different from Area 1 in which the major portion of flow occurred in the existing canal and the area adjacent to it.

Conservation Area 3A covering an area of 786.6 square miles is also within the confines of the Everglades. Ground elevations range from 13 ft. msl in the northwesterly portion to 7.0 ft. in the south end. The regulation varies from 9.5 ft. to 10.5 ft. During the dry season, the water storage is largely in the lower southern portion of the area. The construction of Canal 123 traversing the area between pump station S-8 on the north to the south New River Canal enables the more efficient delivery of water to Everglades National Park from

Lake Okeechobee during periods when storage in the south end is inadequate to meet Park demands.

The network system of Area 3A contains 20 nodes and 61 channels as shown in Figure 3. Inflow points of Area 3A are S-8, S-140, S-190, S-11, S-150, and S-9. There are two major openings with an appreciable amount of undefined runoff flowing into the area. One of the major openings is located at the northwest corner of the area along L-2, L-3 and L-4. The other opening is located between L-28 and the L-28 tieback levee which extends for 7 miles. No data was available for S-140, and S-190 prior to September 11, 1969. Junction No. 1 was assumed as the inflow point for backpumping from the Tamiami Canal basin. Outflow points in Area 3A are Junctions 1 and 6 which supply water to ENP and Miami Wellfield. Rainfall stations located within or around Area 3A are S-8, S-7, 3-3, WMD #3-26, S-9, L-28, 3-4, L-67A-4, S-12D and 40 Mile Bend.

FORMULATIONS

The distribution of the water through the given link-node system of conservation areas is sought by the simultaneous solution of two hydrodynamic equations (known as the equation of motion and continuity equation).

The equation of motion for our one-dimensional link system is written as

$$\frac{\partial V}{\partial t} = -V \frac{\partial V}{\partial X} - g \frac{\partial H}{\partial X} - F_f + F_w \quad (1)$$

where:

V = velocity in ft/sec

t = time, sec

X = distance along the link, ft.

H = water surface elevation, ft. msl.

g = gravitational acceleration

F_f = Acceleration due to fluid resistance

$$= \frac{gn^2 V |V|}{2.2 R^{4/3}}$$

n = Manning's roughness coefficient (sec/ft)

R = Hydraulic radius (ft)

F_w = Acceleration due to wind stress

= negligible in our study areas

The continuity equation for each node is given as follows:

$$A_j \frac{\partial H_j}{\partial t} = \sum_{i=1}^K Q_i + Q_j \quad (2)$$

where:

A_j = the water surface area of the node j's

H_j = the mean elevation of the water surface in node j, (ft. msl.),

Q_i = water importation rate to the junction j

K = number of incoming links

Numerical solution of Eq. 1 and 2 entails rewriting of both equations in finite difference form. The initial value of various parameters at time "t₀" is used to determine the rate change of flow and water head during a short interval of time (Δt). Based on the rate change, the next value is computed with the modified Range-Kutta technique (Caraha, 1969) and the whole procedure

is repeated. The interval of integration is divided into four and applied in sequence with the advantage that the intermediate time-step computations improve the stability and accuracy of the model. The velocity, discharge and stage are computed for each time step and each node and each link to provide the spatial and time distribution of the hydraulic regime (in terms of velocity, discharge and stages). The time step is governed by the following criteria as derived by Garrison et al (1969).

$$\Delta t \leq \frac{\alpha \Delta X}{\sqrt{gh}}$$

where:

Δt = unit time step in seconds,

ΔX = length of the link in ft.

α = proportional constant (0.75)

\sqrt{gh} = celerity of wave, (ft/sec)

The concept of the model, its theoretical development, geometric representation, solution procedure, and computer programs are described in the EPA's report (1971). Other modifications made to the model are described below.

Concept of Parallel Channel:

To better define the hydrodynamics of flow through vegetated marsh area and through the existing canal systems, the concept of parallel channel, originally advocated by Heaney and Huber (1972), is applied to certain nodes where actual channels are located. This concept defined more closely the simultaneous flow through the canal and the overland flow through the heavily vegetated area with different velocities.

Concept of Dual Elevation:

Two bottom elevations for each node are included in the model to be consistent with parallel channel concept. The first elevation refers to ground

elevation at each node, and the second elevation refers to minimum stage which can be reached at the node. For a marsh area, the minimum stage would be a ground elevation. For an existing channel the minimum stage would be the crest elevation of the spillway at the outlet point. The bottom elevations of the actual channel are much different from the crest elevation.

Energy Gradient Through Marsh Area:

The The conservation areas include a greater portion of the Everglades which is often described as a "river of sawgrasses". During periods of abundant rainfall it is a floodplain. When rainfall is deficient it is often swept by fires which may even consume the peat soil. The water movement through these heavily vegetated marshlands is extremely slow. Diking around conservation areas has changed the flow characteristics of the basin. During dry conditions the water stage is comparably low; the stage in the existing channel differs considerably from the higher elevated marsh area. The operation of water control structures may cause flow in marshy areas ~~to~~ towards or away from the outlet structure? Since the bottom elevation of the existing canal is much lower than the marsh area, a discontinuity exists between overland flow and flow in the existing canal. In order to generalize such conditions, the estimation of energy gradient between two nodes is based on the following criteria: a) the water stage is first checked with the ground elevation of the two nodes; if the water stage is above the ground elevations; then the head difference of the two nodes divided by the distance between the two nodes is considered as the energy gradient for the channel; b) if the water stage is below either one of the ground elevations then one-half of the difference between the water stage at one node and the ground elevation of the other node (i.e. dry) divided by the distance between two nodes is used for energy gradient of the channel; c) the energy gradient

for the flow through the marsh area is defined

$$S_f = \frac{n^2 V |V|}{h^{4/3}}$$

where

S_f = Energy gradient

n = Manning's roughness

h = flow depth (ft)

V = velocity (ft/sec)

Channel Width for Hypothetical Channel

In the EPA's 1971 report, the width of a hypothetical channel is defined as the distance between two links parallel to the channel and passing through the centroids of the two adjacent triangles sharing the same link. The area of a triangle is equal to one-half of the length between two nodes times the vertical distance between the centroid point and the link of two nodes. This vertical distance has been used as the width of the hypothetical channel in the EPA's model. However, this width has been further reduced by half in this study to provide a greater accuracy in estimation of equivalent depth and volume. (Fig.4).

Surface Area of Nodes

Each node is associated with its area. A Monte Carlo technique developed by Shih (1975-1976) is used to compute the areal coverage of each node. The Monte Carlo technique requires much less time and is a more powerful tool than conventional techniques.

Rainfall Input

The original model assumed a uniform rainfall input over the entire study areas. This is inappropriate for the conservation areas due to the wide spatial and temporal variations in rainfall. The weighting factor based on a modified

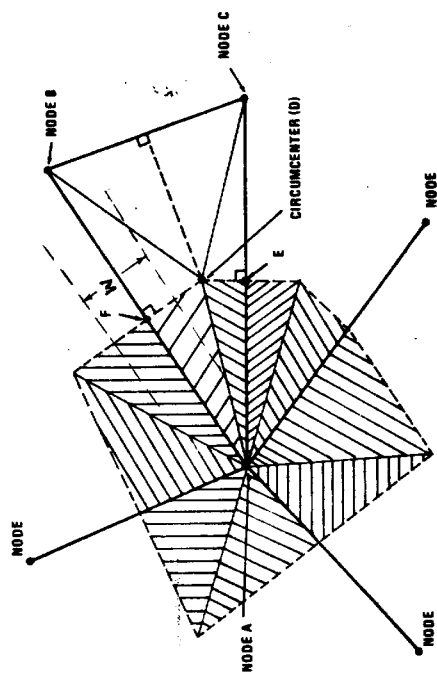


Figure 4. Assignment of Surface Area and Channel Width (w) to an Illustrative Node A

Thiessen polygon of Shih and Hamrick (1975) is used to estimate the rainfall input for each node using the available vicinity gages.

Seepage Rate

A constant rate was used in the original model, however, the seepage rate increases with the depth of water in the study areas. Therefore, a seepage function related to the water stage in the Conservation Areas was developed. When the stages are low and water is confined in the existing borrow canal along the levee, then a certain percentage (10%) of normal rate is applied in the model. A mathematical equation of the seepage function for different water stages is used in the model.

Manning's Roughness Coefficient

Manning's roughness coefficient for grassed channels is known as the retardance coefficient. The retardance coefficient in Conservation Area 2A was reported by the Corps of Engineers (COE, 1954) as shown in Figure 5. Since the vegetation types and their distribution might have changed considerably since then, this set of data was used as a guideline for Manning's n in the study area. In a recent study by Shih and Hallett (1974) on the same subject for the upland marsh area of Chandler Slough in the Kissimmee River Valley (Figure 5) the values for upland marshes is lower than that reported by the Corps of Engineers. However, those two sets of data were used in the model during calibration. Mathematical relations between n -values and flow depths are developed and used in the model.

INPUT DATA REQUIREMENTS

Generally two major types of data are required; they include geometric and hydrologic data types.

A. Geometric Data

This set of data is required by the first program called INDATA and includes:

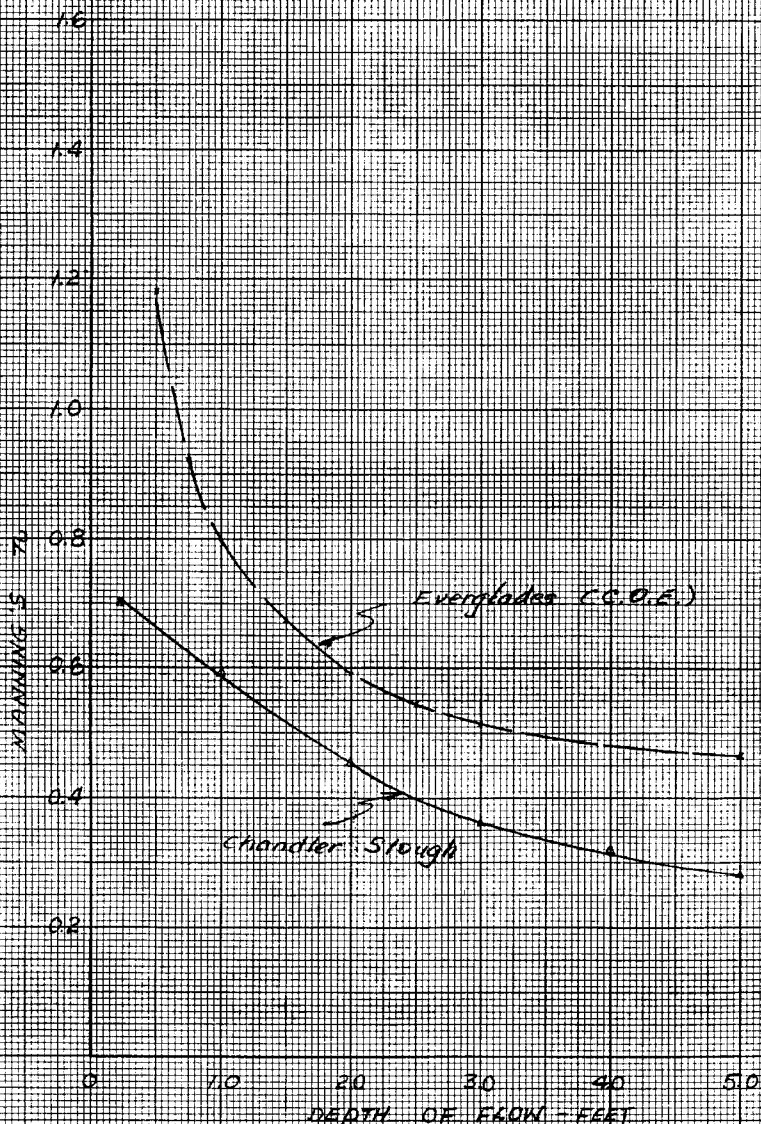


FIGURE 5. Manning " n " in Conservation Area 2A as compared to Sh. H. Study in Chandler Slough.

- 1) Number of days desired
- 2) The time-step
- 3) The starting time of hydrograph input
- 4) Number of junctions of stormwater input
- 5) Point of rainfall information
- 6) Total number of nodes and channels in the desired network.

At each node (junction), the input requires:

- 1) An assigned junction number,
- 2) Water surface elevation,
- 3) Bottom elevation
- 4) Surface Area
- 5) Junction flow out of receiving water junction
- 6) Manning's coefficient
- 7) Junction x- and y- coordinates

B. Hydrologic Data

The hydrologic data included monthly evaporation, daily rainfall at selected gages and daily discharge at all inflow and outflow locations within the study area. The seepage function and Manning's "n" for marshland are also required by the model as mentioned previously.

ASSUMPTIONS

1. The equation of motion used in the model assumes one-dimensional uniform flow of an incompressible fluid in an open-rectangular prismatic channel.
2. Each cross section of the channels is assumed to have a uniform velocity distribution and a hydrostatic pressure distribution.
3. The water surface is assumed to be horizontal across the cross section.

4. The n-values determined for steady flow are applicable to unsteady flow.
 5. Flow is well mixed.
 6. Historical hydrologic data are assumed for the future conditions due to the lack of reliable projection method.
 7. Characteristics of Conservation Areas are as follows:
- A. Conservation Area 1:

Junction No. 18 in Area 1 was assumed to be a new backpumping station near S-5A to avoid excessive inflow with existing pumping Station S-5A. Junction 1 is generally used as outflow point for regulatory flow. However, S-39 is located close to S-10. Therefore, Junction 1 is also used as a water supply point for Service Area 1. Summation of these two has been used as total outflow from Junction 1 in the model.

B. Conservation Area 2A:

Junctions 14, 18 and 17 in Area 2A are inflow points. Inflow from S-10 was split into junction 14 and 18 with junction 17 assumed as backpumping station from service area 2. Junction 1 is also assumed as a water supply station for Area 2A.

C. Conservation Area 3A:

There are two major openings with an appreciable amount of undefined runoff flowing into Area 3A. The two openings are located at the northwest corner of the Area 3A along L-2, L-3 and L-4 and between L-28 and L-28 tie-back levee which extends about 7 miles. S-140 and S-190 flow has been assumed to represent the amounts of flow through these two openings. Junction 1 of Area 3A was assumed to be an inflow point for backpumping from C-4 basin.

EVALUATION OF RESULTS

A. Calibration of the Model

The purpose of the calibration was to demonstrate the model's ability to simulate the real system under various historical conditions, and to develop a set of parameters which can reproduce adequately the historical events of the real system. If such a parameter can be defined, then the outcome that resulted from the model under various backpumping cases would provide reliable information regarding the impacts to the water regime in the conservation areas. Very little research exists to provide a sound basis for describing the flow characteristics in the Everglades, therefore, the calibration of the model was based on the comparison of the model output with observed historical stages at selected gages located in the conservation areas. The calibration of the model was first approached by selecting several wet periods in the area. The set of Manning's coefficients developed by the Corps of Engineers in Area 2A was then applied to check the simulated results with the recorded values. Occasionally the result on one or two of the selected gages did not check closely with recorded values. Other values were then tried. Sometimes, the bottom elevation of the node points had to be adjusted to account for the variations due to the vegetation types, coverage, presence of tree island etc. This type of trial and error approach was found to be the most difficult part in defining the parameters for the model.

Figure 6 & 7 show the simulation results of 1974 in Area 1 and Area 2A at selected gages. The Manning's n values presented in Figure 5 were used in Areas 1 and 2A. The close agreement between the generated and recorded value indicates the validity of the modified model in reproducing adequately the historical pattern of flow distribution in conservation areas. Two sets of Manning's n values were used in Area 2A and their results are plotted with recorded values as shown on Figure 7. The results with a cross mark (x) were

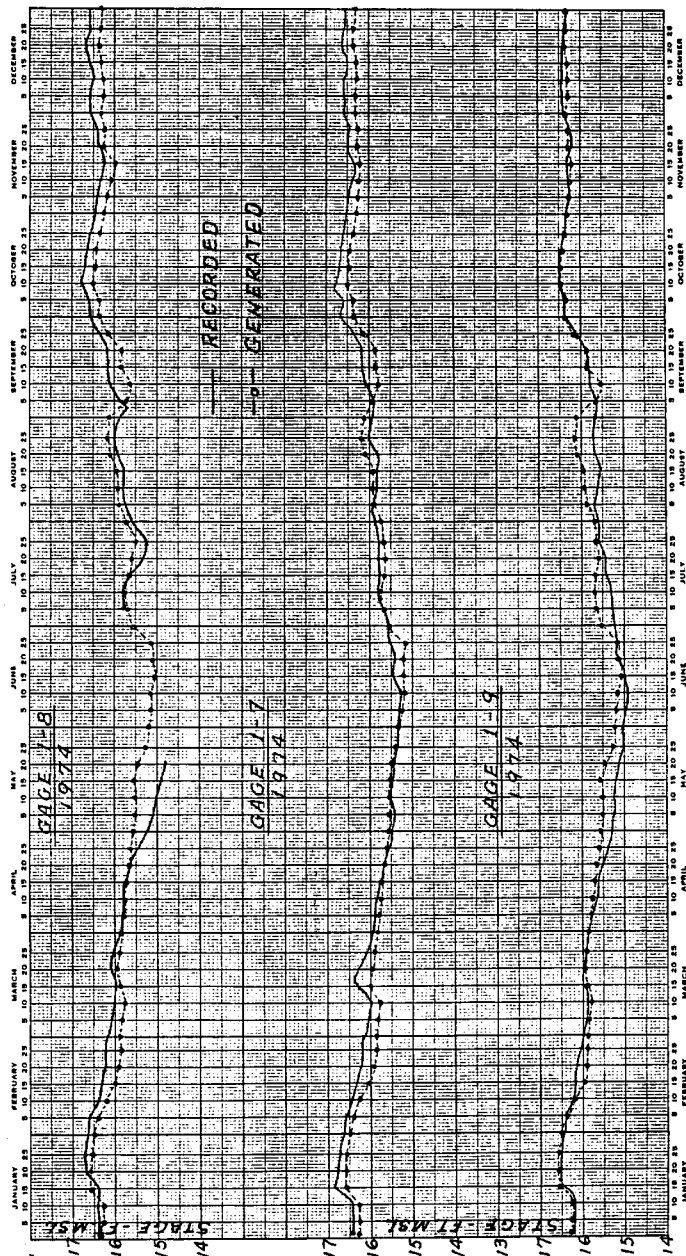


Figure 6. Simulation of 1974 in Conservation Area 1 as Compared to Recorded Values at Selected Stages

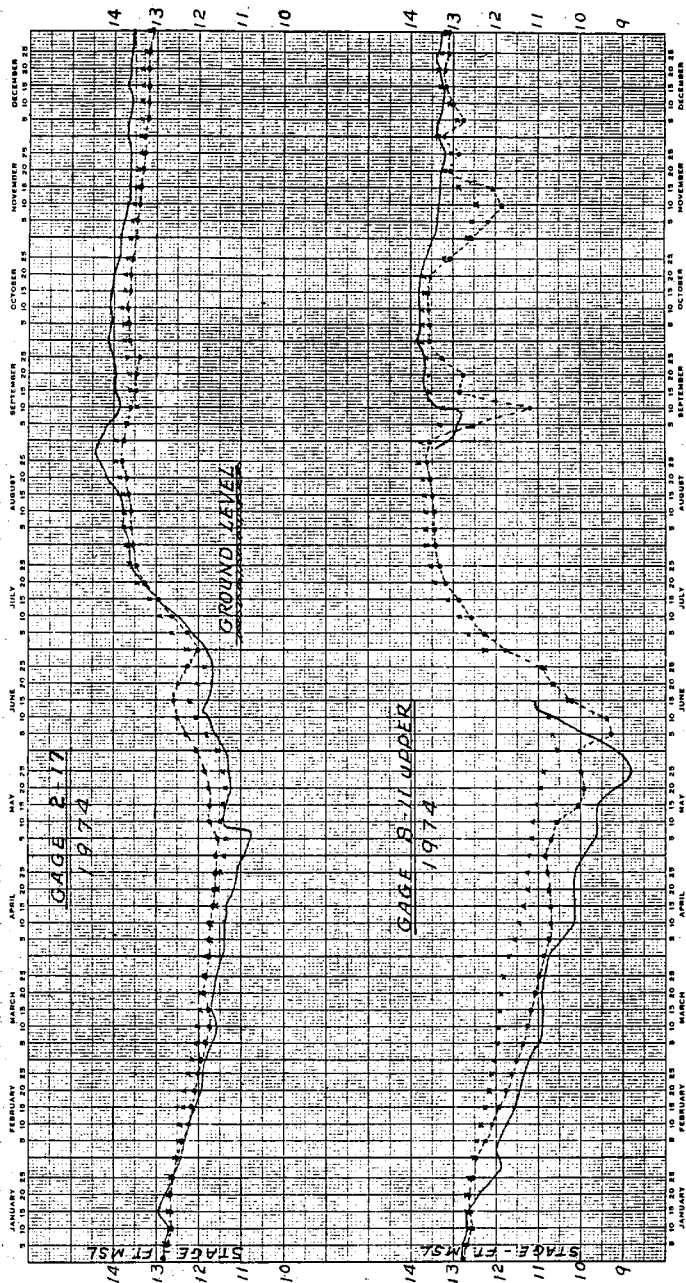


Figure 7. Simulation of 1974 in Area 2A with Two Sets of Manning's "n" Values as Compared to Recorded Values at Selected Sites.

based on Corps of Engineers' value for Area 2A, and the one with a circle mark (o) were based on the values of Shih and Hallett (1975). The later set of values produced much better results for the first five dry months of 1974 for Area 2A; however, the Corps' value produces slightly better results for the wet season of the year. Therefore, the effect of seasonal change to Manning's roughness and flow characteristics is obvious in Area 2A than Area 1. This may also be due to the fact that major flow in Area 1 occurred within the existing canal system rather than in the marsh area, in addition, the water condition in Area 1 were wetter (i.e. most of the marsh area is under water even in the spring of 1974) than Area 2A.

The regulation schedule for Area 2A is shown in Figure 8. If the stage at Gage 2-17 exceeds its regulation schedule, then the gate at S-11 structure would be operated to regulate the water condition. All the historical inflow and rainfall except S-11 flows were inputs to the model. The operation of gates at S-11 structures would be directed by the model according to the generated stage at Gage 2-17 as shown in Figure 8. Figure 8 shows the comparison of generated and recorded stage at Gage 2-17. Figure 9 shows the generated discharge and historical discharge at S-11. From the results indicated in these two figures, the model can be a useful management tool to assess the water distributions in the Conservation Areas under various regulation schedules or assumed storm water

B. Long Term Simulation

Application of the model to generate long term historical conditions for the period from Jan. 1, 1962 through Dec. 31, 1973 was performed in Area 1. The results are presented in Figures 10 and 11. Gage S-6 and 1-8 are gages located in the existing canal system, and gages 1-7, and 1-9 are interior gages located in the central marshland of Conservation Area 1. The circle dots shown on the graphs are the recorded month end stages. This simulation was based on the

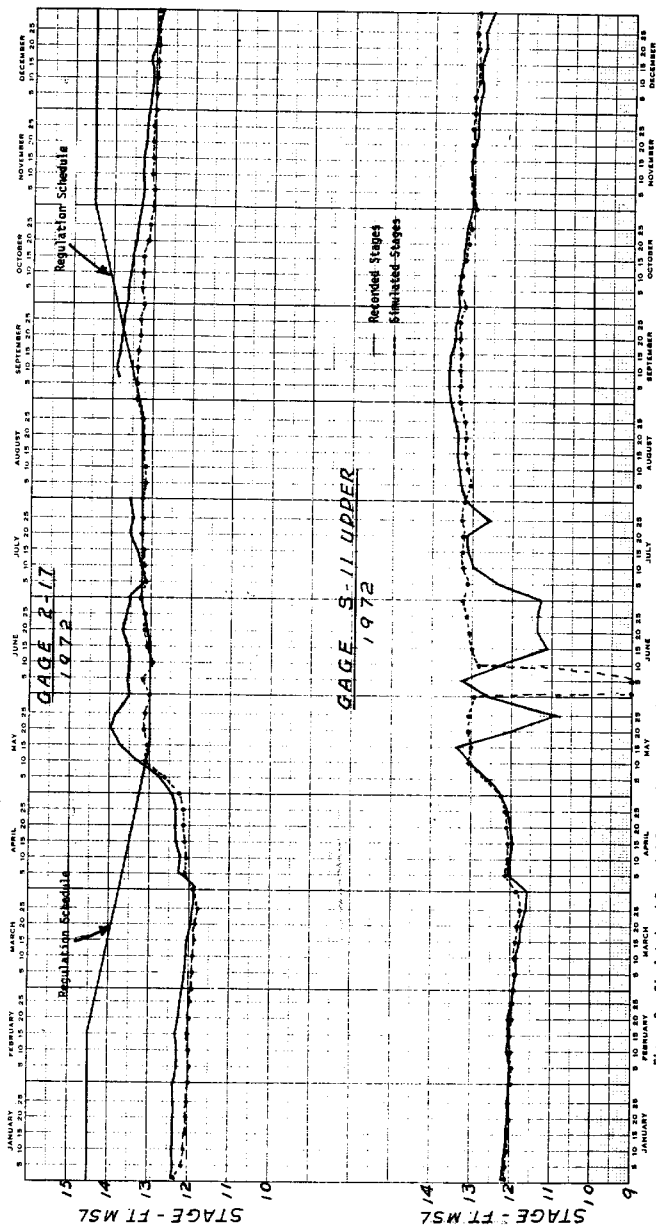


Figure 6. Simulated and Reported Stages for Gages 2-17 and S-11 in 1972

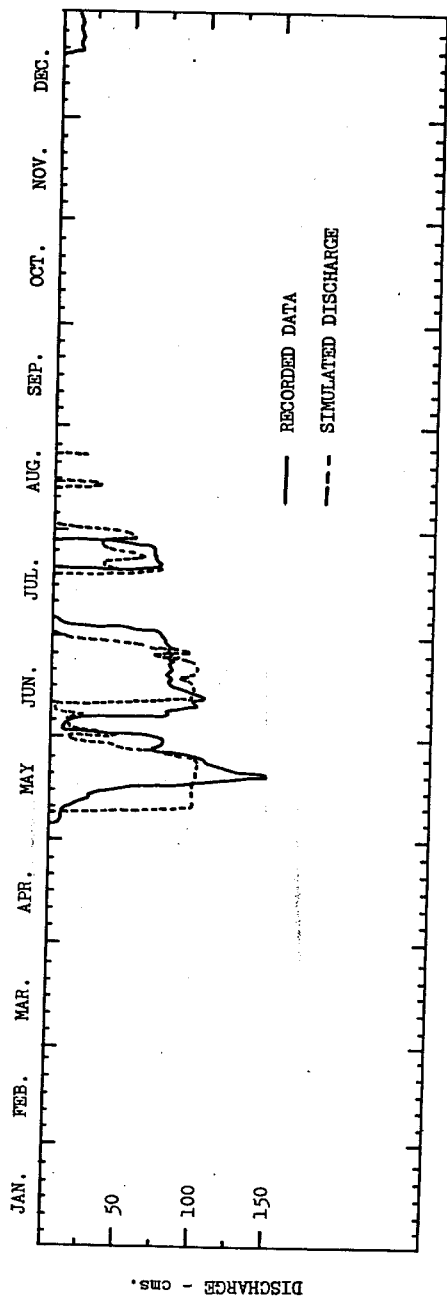


Fig. 9. Comparison of simulated discharge with recorded data at S-11, in 1972.

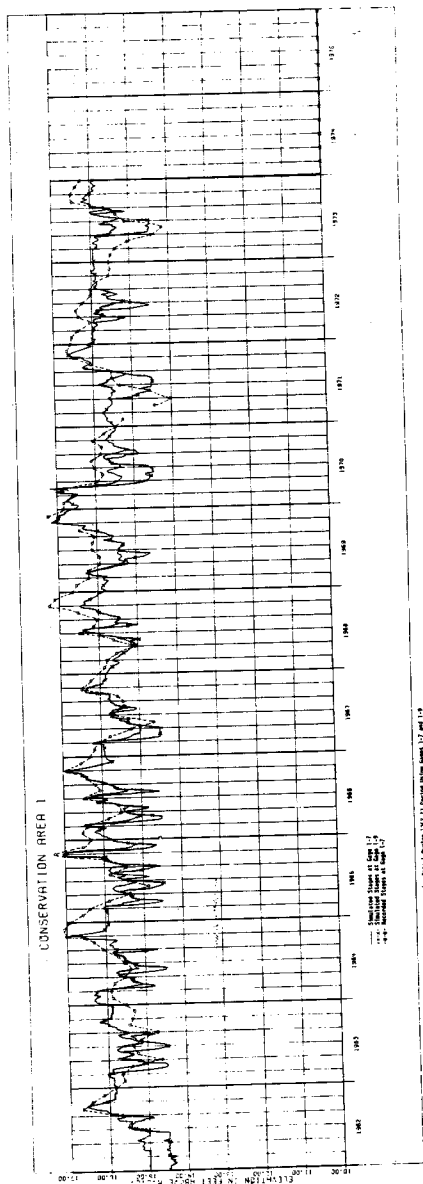


Figure 1. Elevation Results for Conservation Area 1 during 1960-1975 Period (Data from 1960-1975)

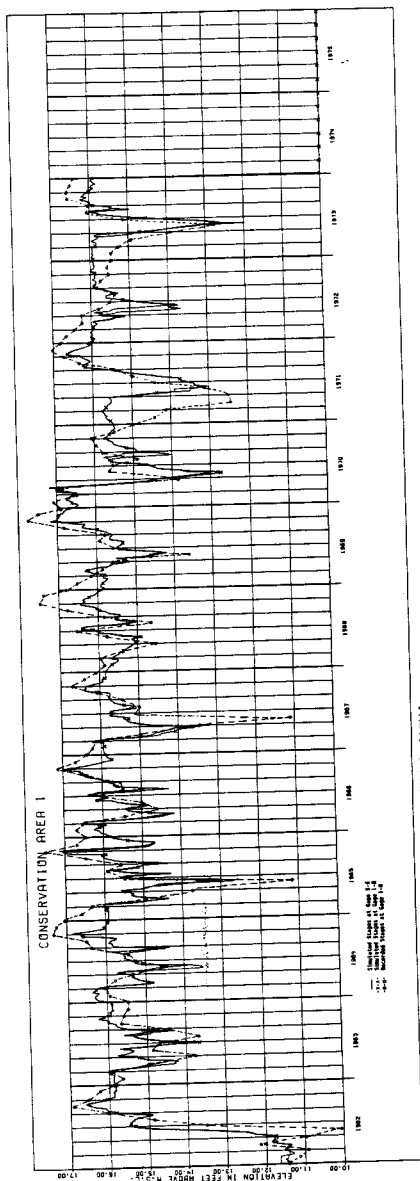


Figure 11. Elevation Station 1 for Conservation Area 1 during 1962-1978. Period of Stationing: 1962-1978.

initial stage on December 31, 1961. All other recorded rainfall, discharge, monthly Et, and seepage function etc. are inputs to the model. Generally speaking, a fairly close relationship exists between the generated and recorded values as can be observed from these plots. The deviation from recorded values for interior gages is far less than the one in the canal system. There are periods in which the generated stage may either be higher or lower than recorded values, these may result from the following reasons:

- a. Only six raingages in the area are available to cover the rainfall characteristics of South Florida.
- b. The reliability of the historical flow through the water control structures such as S-5A, S-6, S-10 etc. is a variable. The computed flow for these structures are based on different rating curves. These rating curves are off from $\pm 8\%$ and more; for example, only three field measurements were available during 1970 and 1971. The current rating was based on these three measurements which are still far from sufficient to determine the amount of flow through S-10. In addition, the current rating is much different from the one used in 1960 as shown in the COE DDM dated 1960. The discharge data was never updated and recomputed.
- c. Lack of Manning's n values as a function of vegetation and seasonal changes. The Manning's n values also varies with flow rates. The value of 0.03 is used in canal flow.
- d. The assumption of only one outflow point may not reflect the actual field conditions. There are three existing structures at S-10. They are S-10A, C and D with approximately two miles apart. Generally, only one structure (S-10A) was operated, except during high flow condition other structures may operate. Therefore, a second outflow point (Node Point #2) was used if the flow at S-10 exceeds 4000 cfs. This approach

did help to a certain degree to the result of simulation; however, a closer network with a detailed flow information on each structure should be developed to reflect the real system better.

- e. Errors may accumulate and increase over the years. The results for selected monthly periods are checked within a range of 0.20 ft. with the recorded values. However, the difference more than 0.2 ft. were resulting from a longer term simulation. The error may be caused by the above mentioned factors. In order to eliminate the possibility of accumulated errors in the model, annual simulations were made (i.e. the length of simulation is limited to one year). The results of this type of simulation are shown in Figure 12. Some improvements can be observed from the plots, particularly for the year 1974. The parameters developed for the year 1974 were then applied to other years.

Figure 13 shows the simulated results for gages S-38 and 2-17. Gage S-38 is a canal gage, and gage 2-17 is an interior gage located in the center marshland of Area 2A. The generated stages follow the general pattern of the recorded values fairly well for some of the years. However, the generated stages deviated from recorded values generally over 0.20 ft. The results for the early months of 1969-70, 73 and 74 tended to be either lower (1969, 1970) or higher (1973 and 1974) when compared to historical stage. However, the simulated stages for the wet months are generally better. As presented in the calibration run in Area 2A a different set of Manning's n should be applied in the model to cover the change of n -values due to seasonal effects. Since the marsh flow in Area 2A is the dominant factor of the entire flow system, the following reasons may contribute to the poor simulation in Area 2A:

- a. Gage 2-17 is the only interior raingage in this 175 sq. miles marsh area. Therefore, the estimation of rainfall input to the system is very approximate.

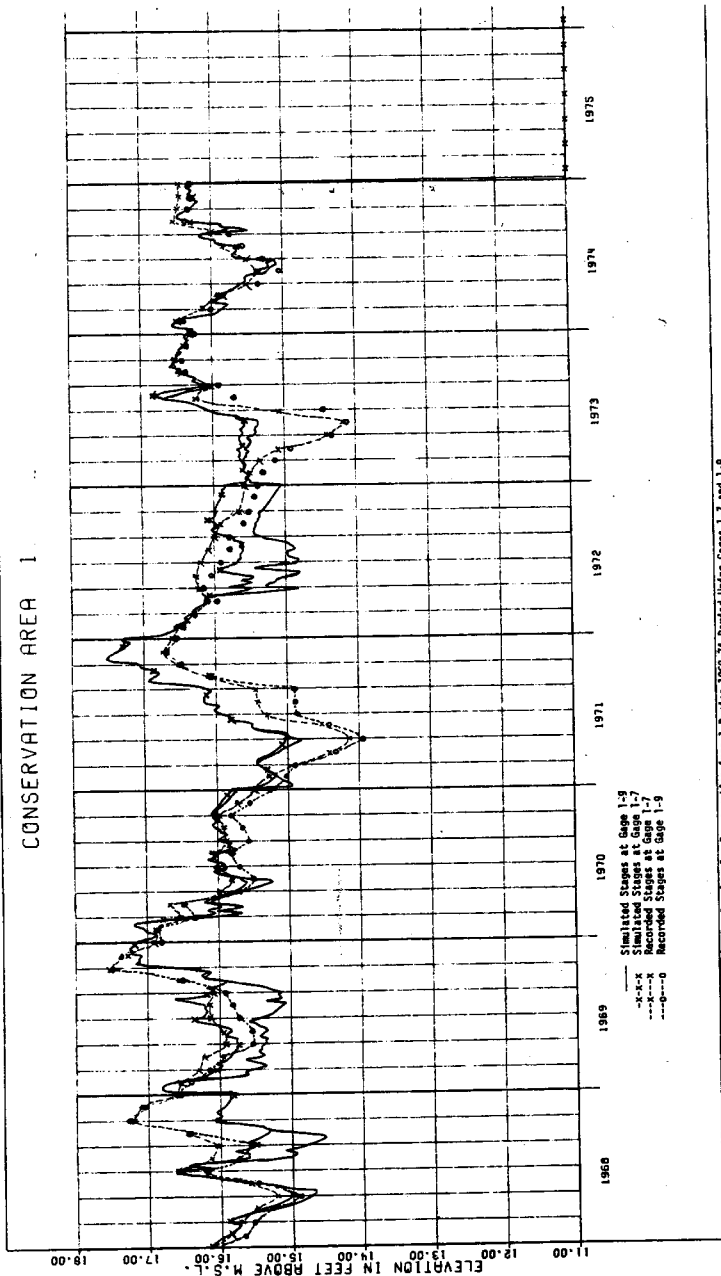


Figure 12. Simulation Results for Conservation Area 1 During 1968-74 Period Using Gages 1-7 and 1-9

CONSERVATION AREA 2A

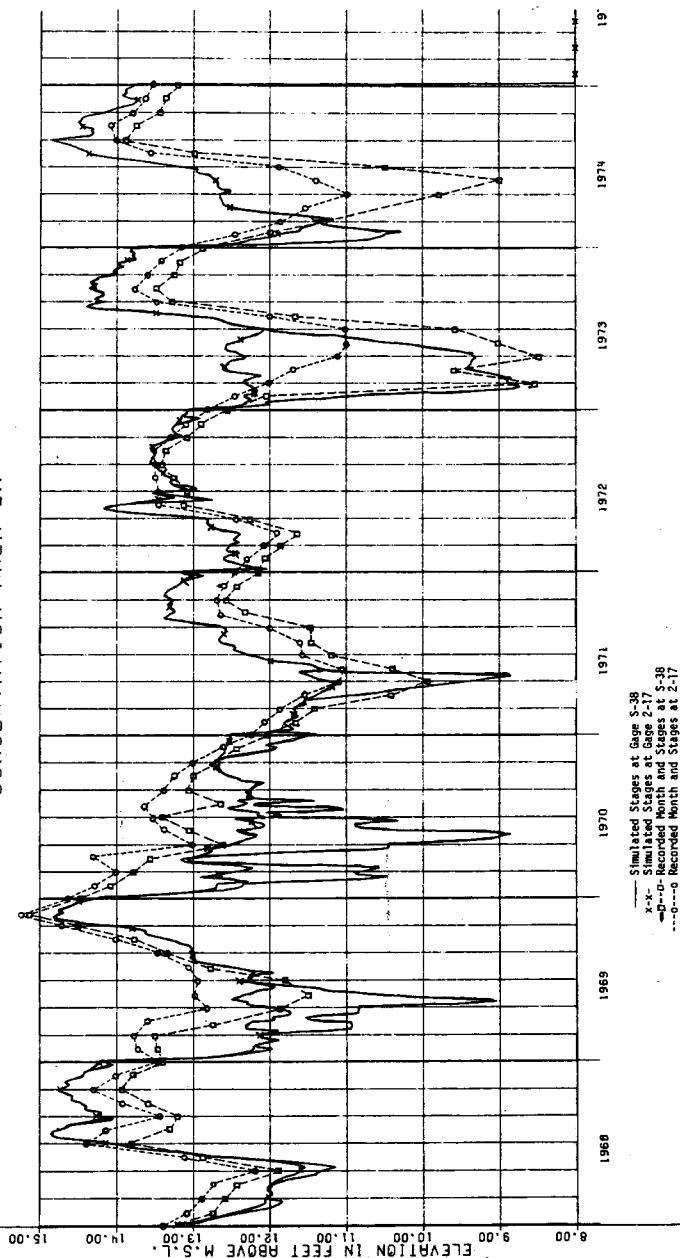


Figure 13. Simulation Results for Conservation Area 2A During 1968-74 Period Using Gages S-38 and 2-17

- b. The discharge values at S-11, 7, 10, need to be verified.
- c. The variation of Manning's n with flow rate should be further investigated.

Figures 14 and 15 show the generated stages at gages 3-2, 3-4, 3-28 and 3-3 as compared to the recorded month end data. The generated results were based on the initial stage on December 31, 1968 and continued through December 31, 1974 with historical daily flow and rainfall data at those selected raingages. Due to the following reasons, the simulation was not started before 1969,

- a. The levee material was directly taken from the borrow canal along L-67A and L-68. Therefore, L-67A and L-68 borrow canal was not a water delivery canal to meet the Everglades National Park demands not until mid 1968. The improvement and enlarging of this borrow canal was done during 1966 through 1968.
- b. The Miami Canal in Conservation Area 3A (i.e. C-123) was under improvment until Dec. 1969.
- c. Pump station S-140 was not activated until Sept. 1969 and the flow from northwest corner of Area 3A along L-1, L-2, L-3, and L-4 was not available prior to Sept. 1969.

The flow characteristics in Conservation Area 3A prior to mid 1969 were not stable due to these changes mentioned. The geometric information for all existing channels in Area 3A were based on the latest as-built map of these canals. The set of n-values based on the simulation results for the year of 1974 was not suitable for rest of the years. Therefore, two sets of n-values had to be used in the simulation (see Fig. 5). One set of n-values was applied to normal or below normal rainfall years, and the other set of n-values was applied to wet years such as 1969 and 1970. The n-values for yearly low or

CONSERVATION AREA 3A

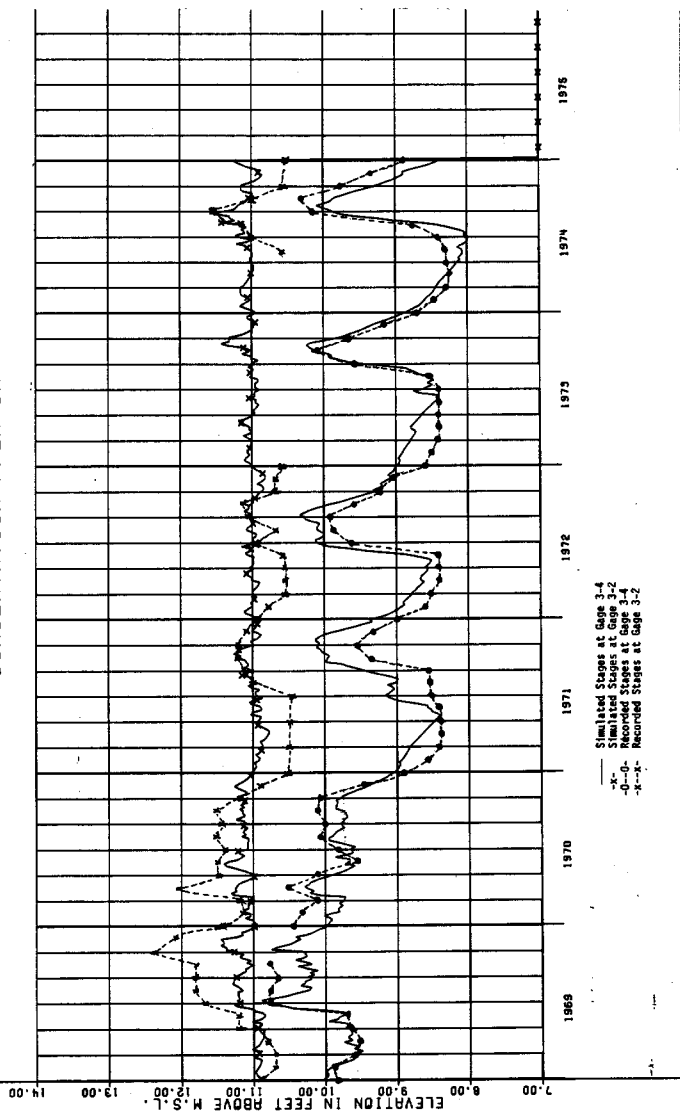


Figure 11. Simulation Results for Conservation Area 3A During 1969-74 Period Using Gages 3-4 and 3-2

CONSERVATION AREA 3A

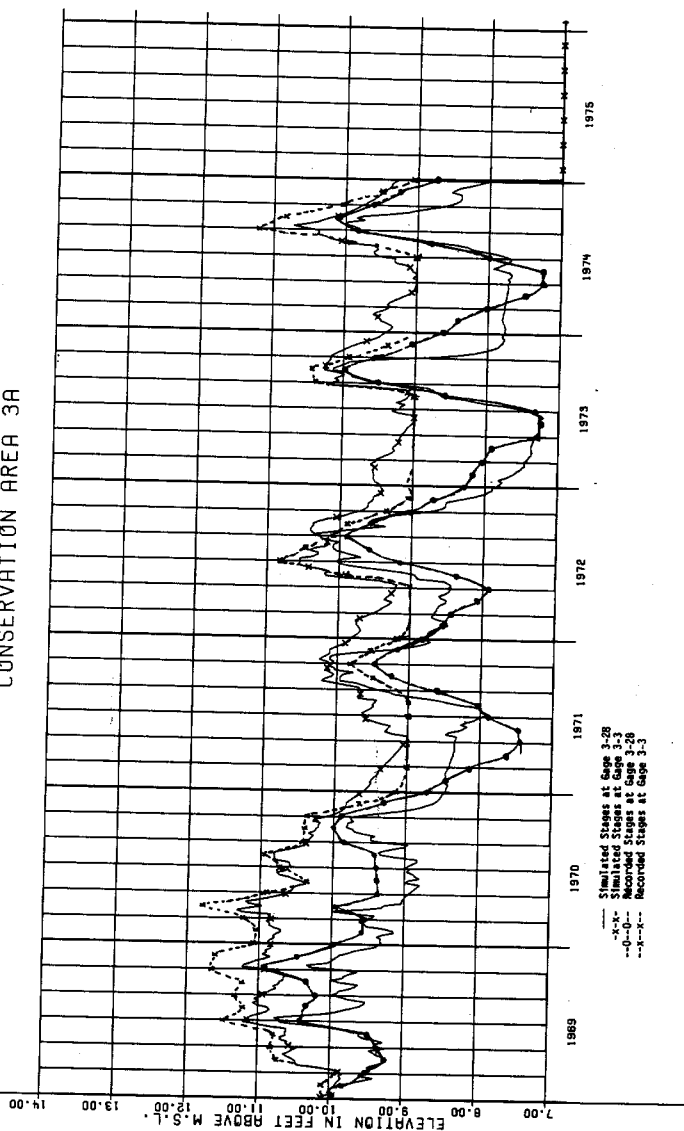


Figure 15. Simulation Results for Area 3A 1963-74 Period Using Gages 3-28 and 3-3

normal periods are slightly lower than the wet year values. This may be due to the frequent fires in the Everglades during dry months such as the fire that occurred during the spring of 1971 and 1973.

Simulated results for gages 3-28, 3-3, 3-4, and 3-2 follow the recorded values fairly good except those months when the water was near or below ground surface. For example, the simulated results for early 1971 is higher than recorded values at gages 3-3 and 3-2. These two gages generally respond to local rainfall more than canal flow or inflow from water control structures such as S-11, S-140, or S-8 etc. (The influence occurred only during high water conditions). Therefore, the amount of rainfall input to these nodes becomes very important. The results at gages 3-3 is generally much better than gage 3-2. This may be due to the fact that no rainfall recording device is available at 3-2. The simulation at gage 3-4 is excellent except the summer of 1971. The simulation at 3-28 is generally good except for the period of early spring where the generated stage always tends to be higher than recorded values. Yet sometime the generated values at this gage did reach down to ground surface during April and May of 1973, but not in 1971 and 1974. Gage 3-28 generally responds to rainfall, backwater flow from the canal at south end of Area 3A and sometime responds to the overland flow from the opening of L-28. In addition, the higher infiltration rate at this gage may be possible due to the location of this gage which is at the western border of Biscayne Aquifer.

The continuous simulation over a number of years tends to increase the accumulated errors in the simulations. The simulation based on an annual basis was done for the period from Jan. 1, 1968 through Dec. 31, 1971. The period was selected so that a typical wet and dry year could be used in the water quality analysis for the conservation areas as a part of the water use plan for the Lower East Coast. The results based on annual simulation are presented in Figures 16

and 17. The results indicate a general improvement for gages 3-3, 3-2 and part of a 304 but not for 3-28. This indicates a need for improving the model in simulation of the flow through the low lying marsh areas in Area 3A.

C. Application of the Model in LEC Water Use Plans

The simulation for selected typical wet and dry years as mentioned in Item B were applied. Alternative plans included in the Water Use Plan are shown as follows:

- a. Backpumping from C-51 basin alone,
- b. Backpumping from C-51 and Tamiami Canal basins to Conservation Area 3A.
- c. Backpumping from C-51 and Tamiami canal basins to Area 3B
- d. Backpumping from C-51, Hillsboro and Tamiami Canal to Area 3A
- e. Backpumping from C-51 and Hillsboro Canal to Area 2A.

Under each alternative plan, three levels of land use conditions were evaluated:

- a. Present land use (based on 1973 land use)
- b. County Master Land Use Plan (future land use)
- c. Maximum water supply based on maximum backpumping to meet all demand

It was found that the results for the requirement levels under the county master land use plans, and maximum water supply plan were approximately the same as obtained from the optimization model. Therefore, the water requirements under the future land use condition were used in the analysis to represent the requirements of either the county master land use plan or a maximum water supply plan.

Figure 18 shows the scheme of the model with the interactive programs.

D. Other Possible Applications

The receiving water body may be an estuary, a stream, or a lake etc. Since the adapted model did not include tidal effect as boundary condition, the model can be applied to a stream, canal, or a shallow well mixing lake. The model can be used to evaluate any structural or nonstructural changes in

CONSERVATION AREA 3A

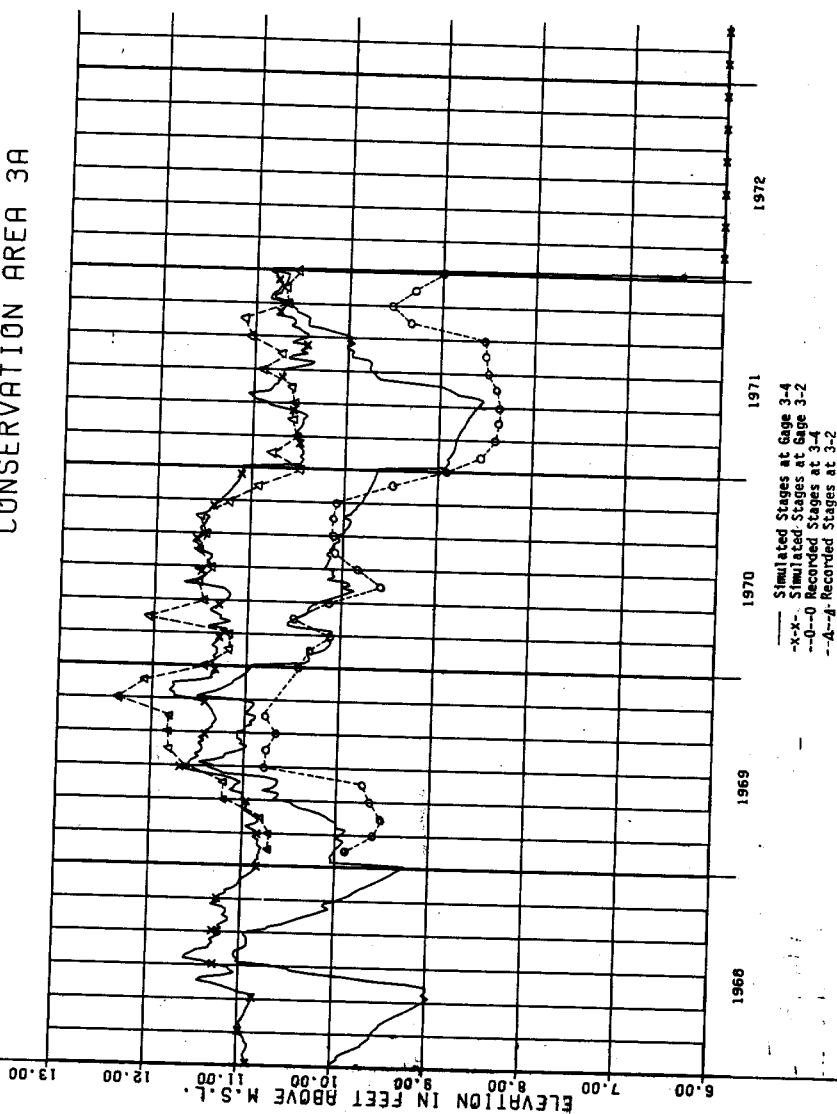


Figure 16, Annual Simulation Results for Conservation Area 3A During 1968-71 Period Using Gages 3-4 and 3-2

CONSERVATION AREA 3A

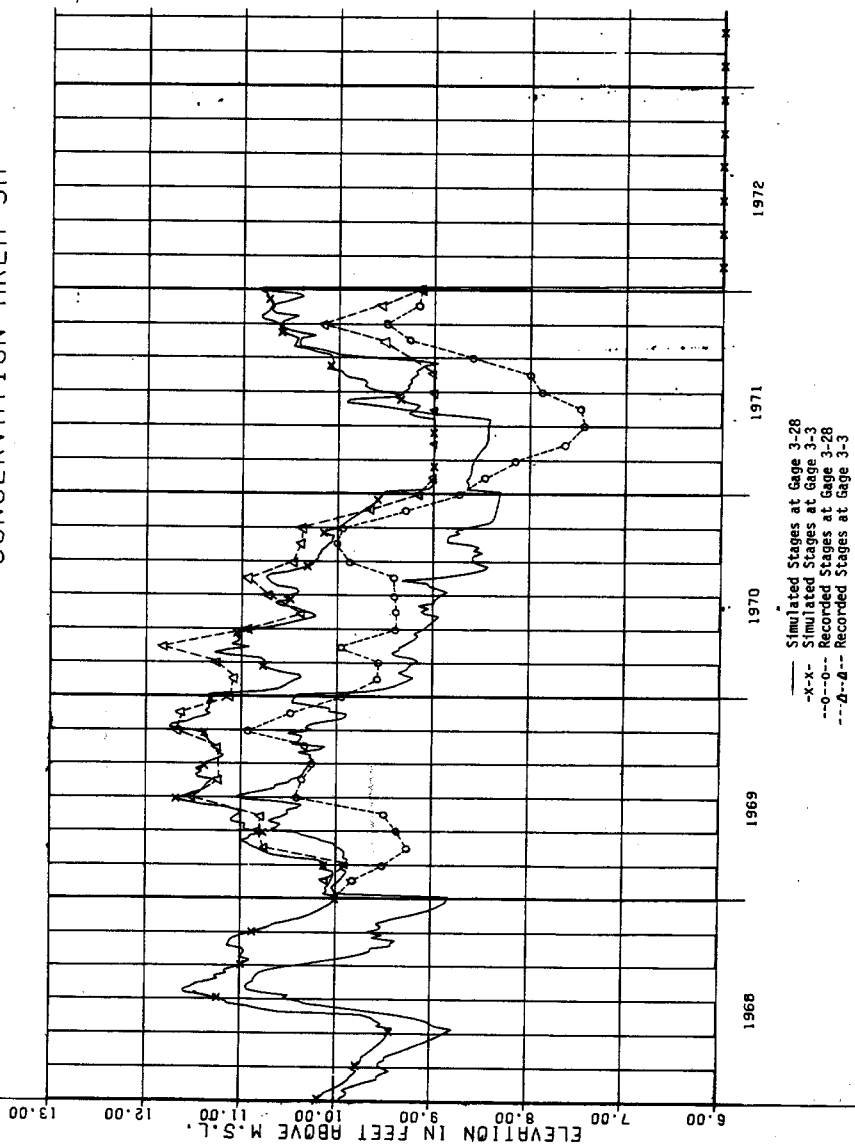


Figure 17. Annual Simulation Results for Area 28. Period: 1968 to 1972.

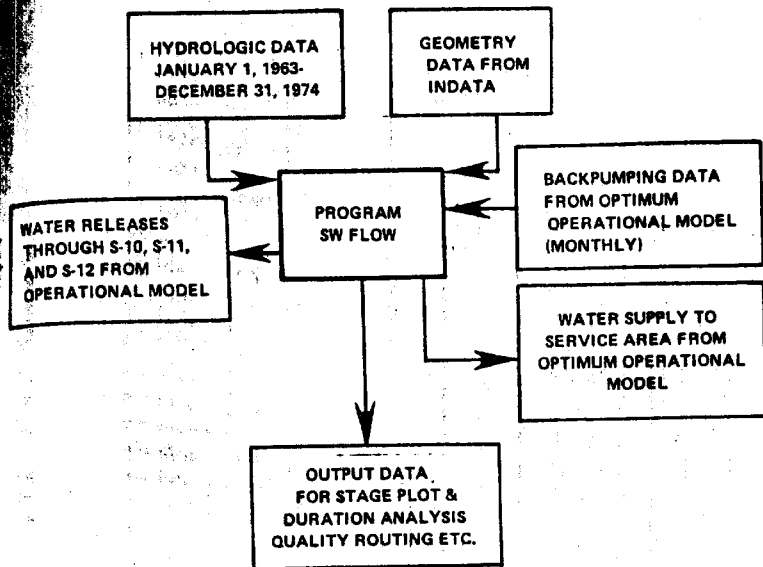


FIGURE 18

Figure 18. An Overall Operational Scheme of the Model

the water systems of the conservation areas. This may include (a) marsh treatment of inflows (backpumping flow) for water quality enhancement, (b) water conveyance due to the additional inflow structure near S-6 to provide inflow to the northern corner of Conservation Area 2 and (c) other flow control structures in borrow canal for better water storage distribution. The applications of the model to the Everglades agriculture or other district areas for the evaluation of water quality distribution resulting from agricultural runoff or urban runoff are also possible.

E. Limitations of the Model

In addition to the assumptions mentioned earlier, some limitations of the model are as follows:

1. Number of nodes and links in the network system is limited to 30 and 91 respectively.
2. No mathematical treatment is given to optimize the parameters of the model.
3. Tidal effects or water structures other than dam or spillway are not included.
4. No bridge losses or losses due to expansion or contraction in the canal are estimated in the model.

CONCLUSIONS:

The concepts of the receiving water quantity model are modified to describe the hydraulic distribution through the marsh and canal system of three conservation areas of the Florida Everglades. The modifications are related to the concept of dual elevation for each node, hydraulic radius, energy gradient for the marsh area, width for the hypothetical channel, use of the Monte Carlo technique to compute area. The modified model is shown to reproduce adequately historical stages and discharge for a short period of one year. However, in some periods, especially during dry years, the generated stage may either be higher or lower than the recorded values. It is expected that the model be improved continually as state-of-the-art improves and additional field data becomes available. Further improvement of the model is possible through the following steps:

1. To provide better representation of the real water body of the conservation areas by increasing the number of nodes in the network system.
2. Existing canal section and topographic elevation in conservation areas need to be refined.
3. Historical flow data at water control structures needs to be updated and to be recomputed through the improved structure ratings.
4. The relationship between Manning's roughness coefficient and the variation of the vegetation communities and their seasonal changes is required.
5. To improve the simulation for low water (or dry year) conditions, a soil moisture accounting model for interior nodes may be helpful.

ACKNOWLEDGEMENTS

This report is based on the work performed under in-house Program 8025 towards the development of the Water Use Plan for the Lower East Coast of South Florida. Thanks are expressed to Barbara Hart, hydrologist, SFWMD for her preparation of daily hydrologic input data for the model. Thanks are also extended to Robert Hamrick and Peter Rhoads for their assistance and suggestions during the development of this model. Dr. Tony Shih is acknowledged for his assistance in application of Monte Carlo techniques and in furnishing the Chandler Slough data. This report is prepared as a part of documentation efforts initiated by Stan Winn, Deputy Director of the Resource Planning Department. The technical editing was done by Ashok N. Shahane of the Water Resources Division.

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9. Shih, S. F., 1976. "Methods of Computing Area", Agronomy Journal, Vol. 68(5):827-829.

APPENDIX: COMPUTER PROGRAM DOCUMENTATION

FLOW CHART

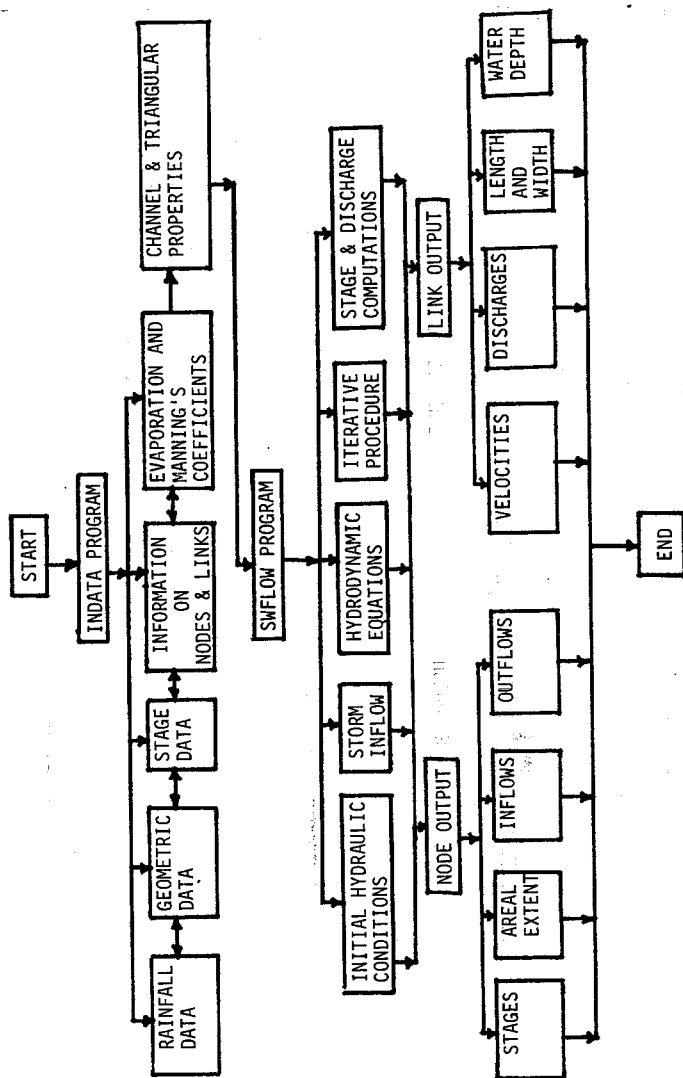
INPUT DATA LIST FOR SWFLOW AND INDATA

PROGRAM LISTING OF SWFLOW AND INDATA

ILLUSTRATIVE INPUT DATA SETS FOR SWFLOW AND INDATA

A SAMPLE OUTPUT

A FLOWCHART OF VARIOUS COMPONENTS OF THE RECEIVING WATER QUANTITY MODEL



INPUT DATA LIST FOR "SFLOW PROGRAM"

INPUT ORDER FOR PROGRAM SWFLOW (CARD INPUT)

Card Group	Format	Columns	Description	Variable Name	Default Value
1	3I5	1-5	Starting month in a year	JMO	None
		6-10	Ending month in a year	NMON	None
		11-15	Backpumping point	IN	0
2	16F5.2	1-5	Mtn. Water Elev. at Node No. 1	FDEP(1)	None
		6-10	" " " " 2	FDEP(2)	None
		:	at last node	:	:
		76-80	Min. Water Elev. at last node	FDEP(NJ)	None
3	16F5.2	1-5	Regulation stage at first day of year	REG(1)	None
		6-10	Regulation stage at 2nd day of year	REG(2)	None
		:	:	:	:
		:	Regulation stage at last day of year	REG(365)	None
4	16F5.2	1-5	Initial stage at first node	H(1)	None
		6-10	" " " 2nd node	H(2)	None
		:	:	:	:
		:	last node	H(NJ)	None
5	20A4	1-80	Title	Title	None
6	3I5	1-5	Conservation Area No.	IRD	None
		6-10	Indicator Gage	IND	None
		11-15	No. of year to be run	ICONT	0
7	16I5		Storm water input control card NJSW		
		1-5	First junction number	JSW(1)	None
		6-10	2nd Junction number	JSW(2)	None
		:	:		
			Last junction number	JSW(NJ)	None
8			If ISWCH(3) = 4 on card 3 of Program INDATA skip to 15		
	3I5	1-5	Total number of time steps not to exceed 31	NTIMST	None
		6-10	Total No. of raingages (Max. 15)	NGAGE	None
		11-15	Units for time of flow (time-step) 0 = seconds 1 = minutes 2 = hours 3 = days	NTCC	0
9	8F10.0	1-10	Time of flow for each time step	TEEM(NTI MST)= TE	None
		11-20	:		
10	16F5.2		Repeat this card for each junction (30)	RCENT	
		1-5	Rainfall weighting factor from first rain gage allocated to junction (NJSW).	(NJSW, NGAGE)	None
		6-10			
		11-15	" " last junction(NJSW)		

Card Group	Format	Columns	Description	Variable Name	Default Value
11	3I5		Same as Card 8		
12	I5	1-5	No. of junction has flow input	LXX	0
13	8F10.0	1-10	Flow rate to junction No. JMM	EXXT	
		11-20		(NTIMST, NJSW)	None
			⋮		None
			Repeat this card for the no. of junction which has flow input		
14	16F5.2	1-5	Rain in inches/day for first raingage	DRAIN	
		6-10	Rain in inches/day for 2nd raingage	(NGAGE)	None
			⋮		
			Rain in inches/day for last rain gage		
15			If ISWCH(3) = 4, Skip to Card Group 17		
	8F10.0		Repeat this card for each time step		
			Input Hydrograph		
		1-10	Time of day, sec.	TE(1)	None
		11-20	Flow volume in cfs for first junction	QE(1,1)	None
		21-30	Flow volume in cfs for 2nd junction	QE(1,2)	None
		⋮	⋮	⋮	
			Flow volume in cfs for last junction	QE(1,NJ)	None
16	F10.0	1-10	Terminate input hydrograph cards with TE(1) beyond expected time of analysis		None
17	2A4	1-8	Write ENDQUANT This is final data card		None

INPUT DATA LIST FOR "INDATA PROGRAM"

RECEIVING WATER BLOCK CARD DATA
INPUT ORDER FOR PROGRAM INDATA

Card Group	Format	Card Columns**	Description	Variable Name	Default Value
1	15A4	1-60	Run Title Card	ALPHA	None
2	15A4	1-60	Storm Title Card	Title	None
3			Control Switches		
		1-5	=0, System is influenced by down-stream head relationship (dam)	ISWCH(1)	0
	10I5		=2, System has specified outflow		
		6-10	=0, Print input channel and junction data	ISWCH(2)	0
			=1, skip printing of input channel and junction data		
		11-15	=4, QE(NJSW,2) is computed	ISWCH(3)	0
		16-20	=1, Triangles are used in card group and different Manning's are desired for each leg of the triangle	ISWCH(4)	0
		21-25	=1, Parallel channels are used	ISWCH(5)	0
		26-30	=0, and triangles are used in card group 11 junction surface area must be left out of input data card group No. 9	ISWCH(6)	0
			=2, and triangles are used in card group No.11, junction surface area must be furnished card group No. 9		
		31-35	=1, Manning's coefficient for channels is computed.	ISWCH(7)	0
		36-40	Not used	ISWCH(8)	0
		41-45	Not used	ISWCH(9)	0
		46-50	If ISWCH(S) =1, then channel numbers greater than this number (ISWCH(10)) are parallel to other lower numbered channels.	ISWCH(10)	0
4	I5	1-5	Number of day cycles desired	NTCYC	None
	4F5.0	6-10	Number of hr/day cycle	Period	None
		11-15	Length of hydraulic time step, hr.	QINT	None
		16-20	Length of hydraulic time step, sec.	DELT	None
		21-25	Initial time for start of hydrograph input from cards, hr.	TBERO	None
	3I5	26-30	Number of junction for time-history printout	NHPRT	None
		31-35	Number of channels for time-history printout	NQPRRT	None
		36-40	Number of plots desired	NPLT	0
	3F5.0	41-45	Evaporation, in./mo.	EVAP	0
		46-50	Wind velocity, mph.	WIND	0
		51-55	Wind direction, clockwise, degrees from North.	WDIR	
	4I5	56-60	Day cycle where printed output will start	NQSWRT	None
		61-65	Number of junctions of storm water input from cards	NJSW	None

*Card columns that are not included in the table are "not used"

Card Group	Format	Card Columns	Description	Variable Name	Default Value
5	8F10.0	66-70	Number of points of rain information	INRAIN	None
		71-75	Junction number where a head relationship is specified.	JGW	None
			If INRAIN=0, skip rain input cards 5		
		1-10	Rate of precipitation in./hr.	RAIN(1)	None
		11-20	Time from start of storm, min.	INTIME(1)	None
6	8I10	21-30	" " " " " "	RAIN(2)	None
		31-40	Etc., up to INRAIN points (<50)	INTIME(2)	None
			Junction selected for stage-history printout, NHPRT values (up to 30)		
		1-10	First junction number	JPRT(1)	None
		11-20	Second junction number	JPRT(2)	None
7	8I10	:	:	:	
			Last junction number	JPRT(NHPRT)	None
			Channels selected for flow print, NQPT values, 8 per card (max = 90)		
		1-7	Lower junction no. (numerically lower) at end of first desired channel.	CPRT(1)	None
		1-8	Higher junction no. (numerically higher) at end of first desired channel		
8	8F10.0	11-17			
		18-20			
		:	Lower junction no. (numerically lower) at end of last desired channel.	CPRT(N)	None
		:	Higher junction no. (numerically higher) at end of last desired channel.	QPRT)	
		:			
9	F10.0	1-10	WEIR factor	A1	None
		11-20	Elevation of top of WEIR (ft.),	A2	None
		21-30	Power law for WEIR	A3	None
			REPEAT CARD 9 FOR EACH JUNCTION (90)		
			JUNCTION CARDS		
10	F5.0	1-5	Junction number	J	None
		6-10	Water surface-elevation (ft.) referenced to datum plane.	HEAD(J)*	None
			*****If INTEMP(3) on Card is supplied*****		
			LEAVE SURFACE AREA BLANK.		
		11-20	Surface area of junction (millions of sq. ft.)	AS(J)= SURF	None
11	2F5.0	21-25	Junction flow into receiving waters(cfs)	QIN(J)= QF1	None
		26-30	Junction flow out of receiving waters(cfs)	QUO(J)= QF2	None
		31-40	Junction depth (ft)**	DEP(J)DT	None
		41-50	Junction Manning's coefficient	COF(J)= CF	None
12	20X 2F5.0	71-75	Leave columns blank		
		71-75	X-coordinate (thousands of ft.)	X(J)=X1	None
		76-80	Y-coordinate (thousands of ft.)	Y(J)=Y1	None

*Head is negative when below datum plane.

**Depth is distance to bottom from datum plane (downward is positive).

Card Group	Format	Card Columns	Description	Variable Name	Default Value
10	15	1-5	To terminate junction cards, write 77777		
11			REPEAT CARD 11 FOR EACH CHANNEL OR TRIANGLE (Max. = 99)		
	5I5	1-5	Channel or Triangle Cards	N	None
		6-10	Junction at lower end of channel (Numerically lower)	NTEMP(1)	None
		11-15	Junction at upper end of channel (Numerically higher)	NTEMP(2)	None
		16-20	Blank unless program is to develop geometric data through the use of triangles. Then NTEMP(1),NTEMP(2), NTEMP(3) are the vertices of the acute triangle. Program will develop channel characteristics.	NTEMP(3)	0
		21-25	Blank unless it is a number of a fourth junction which lies between a pair of previous three junctions. Program will develop geometric data.	NTEMP(4)	0
			If INTEMP(3) is supplied then leave columns 26-80 blank. But if ISWCH(4)=1 on card 3, ALEN=MANNING FOR CHANNEL NTEMP(1) TO NTEMP(2), WIDTH=MANNING FOR CHANNEL NTEMP(2) TO NTEMP(3),RAD=MANNING FOR CHANNEL NTEMP(1) TO NTEMP(3).		
	5F10.0	26-35	Length of channel (ft)	ALEN	None
		36-45	Width of channel (ft.)	WIDTH	None
		46-55	Average depth of channel (ft. refer to datum)	RAD	None
		56-65	Manning's coefficient, n.	COEF	0.018
		66-75	Initial velocity (fps)	VEL	None
12	15	1-5	To terminate channel cards, write 99999		

PROGRAM LISTING OF SWFLOW

\$JOB,8012-304,LIN,45
 \$RAT,841/3006
 \$FET,LIN,CA3,512
 \$RELEASE,ALL
 \$ALLOCATE,300
 \$OPEN,20
 \$FORTRAN,L,X,M

PROGRAM SWFLOW

C		HYDRODYNAMICS PROGRAM	SWFL	2
C		TIDAL OPTION	SWFL	3
C		TYPE DESIGNATIONS	SWFL	57
C			SWFL	58
	INTEGER CPRT,ALPHA,TITLE		SWFL	59
	REAL LEN,INTIME		SWFL	60
C			SWFL	4
C		SPECIFICATION STATEMENTS	SWFL	5
C			SWFL	6
	DIMENSION EXXT(31,30),TEEM(31),RCENT(30,13),DRAIN(15)		SWFL	7
	DIMENSION DQIN(30),DOOU(30),REG(366)			
	DIMENSION IENDER(2),ISN(10)			
	DIMENSION BP(31),ONEW(31),FDEP(30)			
C			SWFL	8
C		CONTROL	SWFL	9
C			SWFL	10
	COMMON TITEL2(20),ISWCH(10)			
C			SWFL	13
C		GENERAL	SWFL	14
C			SWFL	15
	COMMON ALPHA(15)			
C			SWFL	18
C		JUNCTIONS	SWFL	19
C			SWFL	20
	COMMON H(30),HN(30),HT(30),HBAR(30)			
	1, NCHAN(30,12),IPDINT(30,12),AS(30),VOL(30),X(30),Y(30)			
	2, DEP(30),COF(30),QIN(30),QOU(30),QINST(30),QINBAR(30),QOUBAR(30)	SWFL	2	
C			SWFL	25
C		CHANNELS	SWFL	26
C			SWFL	27
	COMMON LEN(90),NJUNC(90,2),B(90),R(90),A(90),AT(90),AK(90)	SWFL	28	
	1, Q(90),QBAR(90),QAVE(90),V(90),VT(90),VBAR(90)	SWFL	29	
	2, FWIND(90),NUMCH(90),NTEMP(12),NCLOS(90)	SWFL		
C			SWFL	32
C		PRINTOUT AND PLOTTING	SWFL	33
C			SWFL	34
	COMMON JPRT(30),PRTH(1,90)			
	1, CPRT(90),PRTV(1,90),PRTQ(1,90),ICOL(12)			
	2, JPLT(30),HPLT(30),TT(10)			
C			SWFL	38
C		STAGE-TIME COEFFICIENTS	SWFL	39
C			SWFL	40
C			SWFL	43
C		STORMWATER	SWFL	44
C			SWFL	45
	COMMON TITLE(15),QE(30,2),JSW(30),RAIN(50),INTIME(50)			
C			SWFL	53
	COMMON QT(30,2),ISW(30)			
C			SWFL	61
C		INITIALIZATION	SWFL	62
C			SWFL	63
	DATA (IENDER=6HENDQUANT)			
C			SWFL	84

```

N5=60
N6=61
N20=20
REWIND N20
N21=0
N22=22
NEXIT=0
DO 70 I=1,90
Q(I)=0.0
VT(I)=0.0
B(I)=0.0
FWIND(I)=0.0
CONTINUE

```

70
C
C
C

SUBROUTINE INDATA CALLED TO
READ INPUT DATA

SWFL 85
SWFL 86
SWFL 87

```

REWIND 23
READ(23) NTCYC,PERIOD,DELT,TZERO,NHPRT,NQPRT,NPLT,EVAP,WIND,WDIR
1,  NQSWRT,NJSW,INRAIN,JGW
READ(23) NQCYC,NHCYC,NJ,NC,A1,A2,A3,A4,A5,A6,A7,NPDEL
DO 150 J=1,NJ
READ(23) I,H(J),DEP(J),AS(J),QIN(J),QOU(J),(NCHAN(J,K),K=1,12)
1,  X(J),Y(J)
AS(J)=AS(J)*10.0**6
150 CONTINUE
DO 160 J=1,NC
READ(23) LEN(J),B(J),R(J),A(J),AK(J),V(J),(NJUNC(J,K),K=1,2)
160 CONTINUE
DO 165 J=1,NJ
READ(23) QINST(J),(IPOINT(J,K),K=1,10)
165 CONTINUE
READ(23) (CPRT(I),I=1,NQPRT)
READ(23) (JPRT(I),I=1,NHPRT)
READ(23) (JPLT(I),I=1,NPLT)
READ(23) ALPHA,TITLE,(NTEMP(I),I=1,4)
READ(23) (ISWCH(I),I=1,10)
WRITE(61,7092) (ISWCH(I),I=1,10)
READ(23) (RAIN(I),INTIME(I),I=1,50)
REWIND 23
DO 4 J=1,NJ
DQIN(J)=QIN(J)
DQOU(J)=QOU(J)
4 CONTINUE
READ(60,15) JMO,NMON,IN
READ(60,2332) (FDEP(J),J=1,NJ)
READ(60,2332) (REG(I),I=1,365)
READ(60,2332) (H(J),J=1,NJ)
DELTQ=DELT*FLOAT(NHCYC)
WRITE(N20) TITLE,ALPHA,NJ,NC,NQCYC,DELTQ,((NCHAN(J,K),K=1,10),
XAS(J),J=1,NJ),(LEN(N),(NJUNC(N,K),K=1,2),N=1,NC)
READ(60,7097) TITLE
WRITE(61,7097) TITLE
READ(60,15) IRD,IND,ICONT
RD=0.0
15 FORMAT(3I5)
REWIND N22

```

C
C
C

FURTHER INITIALIZATION

SWFL 98
SWFL 89
SWFL 90
SWFL 91

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NCGT=ISWCH(10)
DO 175 J=1,NC
IF(J.GT.NCGT) GO TO 175

```

	B(J)=0.5*B(J)	
175	CONTINUE	
	DO 170 J=1,NJ	
	DEP(J)=-H(J)	
170	CONTINUE	
	IYEAR=1	
	LN=JMD	
501	KDAY=0	
50	TEPP=NTCYC*86400.	SWFL 93
	IF(NTCYC.EQ.1)TEPP=PERIOD*3600.	SWFL 94
	IF(JMD.EQ.1.OR.3) TEPP=(PERIOD + 24.0)*3600.0	
	IF(JMD.EQ.2) TEPP=(PERIOD-48.0)*3600.0	
	IF(JMD.EQ.4.OR.6) TEPP=PERIOD*3600.0	
	IF(JMD.EQ.5.OR.7) TEPP=(PERIOD + 24.0)*3600.0	
	IF(JMD.EQ.8.OR.10) TEPP=(PERIOD + 24.0)*3600.0	
	IF(JMD.EQ.9.OR.11) TEPP=PERIOD*3600.0	
	IF(JMD.EQ.12) TEPP=(PERIOD + 24.0)*3600.0	
	NTINT = 0	SWFL 99
	TT(1) = 0.0	SWFL100
	TT(2) = 0.0	SWFL101
	NINREC=1	SWFL102
	NSTEPS = 0	SWFL103
	MJSW = 0	SWFL104
	NQUAL = 0	SWFL105
	TDELT = 0	SWFL106
	WEIR1 = A1	SWFL107
	WEIR2 = A2	SWFL108
	WEIR3 = A3	SWFL109
	DO 220 I=1,12	
220	ICOL(I)=I	SWFL112
	DO 222 I=1,30	
	ISW(I) = 0	SWFL114
	QT(I,1) = 0.0	SWFL115
	QT(I,2) = 0.0	SWFL116
	QE(I,1) = 0.0	SWFL117
222	QE(I,2) = 0.0	SWFL118
	TE=0.	SWFL119
	TEP=0.	SWFL120
	DELT2=DELT/2.0	SWFL121
	TZERO=TZERO*3600.0	SWFL122
	W=6.2832/(3600.*PERIOD)	SWFL123
	EVAP=EVAP/(12.*30.*86400.)	SWFL124
	TOLD=0.	SWFL125
	KRAIN=1	SWFL126
	PREC = 0.0	SWFL127
	T=TZERO	SWFL128
	DO 224 I = 1,NHPRT	
	MJPRT = JPRT(I)	SWFL130
	PRTH (1,I) = H(MJPRT)	SWFL131
224	CONTINUE	SWFL132
	DO 225 I = 1,NQPRT	SWFL133
	MCPRT = CPRT(I)	SWFL134
	PRTQ(1,I) = Q(MCPRT)	SWFL135
	PRTV(1,I) = V(MCPRT)	SWFL136
225	CONTINUE	SWFL137
		SWFL138
		SWFL139
		SWFL140
		SWFL141
		SWFL142
		SWFL143
		SWFL144

READING OF INITIAL HYDROGRAPH
INFORMATION FROM INTERFACING

IF(N21.EQ.0) GO TO 230
REWIND N21
READ (N21) TITEL2

```

WRITE (N6,7097) TITEL2
7097 FORMAT (1H1,(20A4))
READ (N21) NSTEPS,MJSW,NQUAL,TDELT,TZERO,TAREA
WRITE(N6,7093)
7093 FORMAT(///1H ,7X,6HNSTEPS,5X,4HMJSW,6X,5HNQUAL,2X,5HTDELT,5X,
1 5HTZERO,5X,5HTAREA)
WRITE (N6,7091) NSTEPS,MJSW,NQUAL,TDELT,TZERO,TAREA
7091 FORMAT (3I10,3F10.2)
READ(N21) (ISW(L),L=1,MJSW)
WRITE(N6,7095)
7095 FORMAT(///1H ,23HSTORM WATER INPUT NODES/)
WRITE(N6,7092) (ISW(L),L=1,MJSW)
7092 FORMAT(10I10)
READ(N21) TT(1),(QT(L,1),L=1,MJSW)
WRITE(N6,7096)
7096 FORMAT(///1H ,12HTIME-STEP ,4X,4HTIME,7X,36HINFLOWS FROM STORM W
1ATER INPUT NODES/)
WRITE(N6,7094)NINREC, TT(1),(QT(L,1),L=1,MJSW)
7094 FORMAT(I7,3X,9F10.1,/, (20X,8F10.1))
READ(N21) TT(2),(QT(L,2),L=1,MJSW)
NINREC=2
WRITE(N6,7094)NINREC, TT(2),(QT(L,2),L=1,MJSW)
I1=1
I2=2
TTP=0.
230 IF(NJSW.EQ.0) GO TO 235
LDD=JMO-LN+1
IF(IYEAR.GT.1.OR .LDD.GT.1) GO TO 45
READ(60,100) (JSW(L),L=1,NJSW)
45 IF(ISWCH(3).NE.4)GO TO 234
READ(N5,231) NTIMST,NGAGE,NTCC
231 FORMAT(3I5,F5.0)
IF(IYEAR.GT.1.OR .LDD.GT.1) GO TO 3
READ(60,232) (TEEM(KM),KM=1,NTIMST)
232 FORMAT((8F10.0))
DO 2330 JTT=1,NJSW
2330 READ(60,2332) (RCENT(JTT,ITT),ITT=1,NGAGE)
2332 FORMAT(16F5.2)
3 DO 2328 KMT=1,NTIMST
DO 2328 JMM=1,NJSW
BP(KMT)=0.0
QNEW(KMT)=0.0
2328 EXXT(KMT,JMM)=0.0
IF(LDD.GT.1.OR.IYEAR.GT.1) GO TO 351
READ(60,231) NTIMST,NGAGE,NTCC
351 READ(N5,231) LXX
IF(LXX.EQ.0) GO TO 23310
DO 2331 L=1,LXX
READ(N5,231) JMM
READ(N5,232) (EXXT(KMT,JMM),KMT=1,NTIMST)
2331 CONTINUE
23310 TCN=1.
20 NNTTM=1
IF(NTCC.EQ.1)TCN=60.
IF(NTCC.EQ.2)TCN=3600.
IF(NTCC.EQ.3)TCN=86400.
READ(N5,462)(DRAIN(LT),LT=1,NGAGE)
DO 2333 JLT=1,NJSW
SUMGAG=0.
DO 233 JLK=1,NGAGE
233 SUMGAG=SUMGAG+RCENT(JLT,JLK)*DRAIN(JLK)
JNUM=JSW(JLT)

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SWFL145
 SWFL146
 SWFL147

 SWFL151
 SWFL152
 SWFL153

 SWFL157
 SWFL158

 SWFL163
 SWFL164
 SWFL165
 SWFL167
 SWFL168
 SWFL169
 SWFL170

 SWFL172

 SWFL176
 SWFL177

 SWFL179

 SWFL182
 SWFL183
 SWFL184
 SWFL185
 SWFL186
 SWFL187
 SWFL188
 SWFL189
 SWFL190
 SWFL191
 SWFL192

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2333 QE(JLT,2)=EXXT(NNTTM,JLT)+.96*AS(JNUM)*SUMGAG/(12*86400)
TE=TEEM(NNTTM)*TCN
TEE=TEEM(NTIMST)
WRITE(N6,102) (JSW(L),L=1,NJSW)
WRITE(N6,76) JMO
76 FORMAT(5X,5HMONTH,I5//)
C WRITE(N6,105)
DO 2336 I=1,NTIMST
TFIRST=(TEEM(I)*TCN)/3600.
TSECND=TFIRST/24.
C WRITE(N6,106) TFIRST,TSECND,(EXXT(I,J),J=1,NJSW)
2336 CONTINUE
GO TO 237
100 FORMAT(16I5)
234 READ(N5,104) TE,(QE(L,2),L=1,NJSW)
C WRITE(N6,102) (JSW(L),L=1,NJSW)
102 FORMAT(1H1,30HHYDROGRAPH INPUTS TO SYSTEM//1H,24X,16HJUNCTION
1 NUMBER/1H,(17X,10I10))
237 IF(ISWCH(3).EQ.4) WRITE(N6,2337) (JSW(L),L=1,NJSW)
2337 FORMAT(1H1,49HHYDROGRAPH INPUTS TO SYSTEM WITH RAIN ADDED//
1 1H,24X,15HJUNCTION NUMBER/1H,(17X,10I10))
C WRITE(N6,105)
105 FORMAT(1H,8X,4HTIME/1H,14H HOURS DAYS,10X,12HVOLUME (CFS))
TEP=TE/3600.
TEPP2=TEP/24.
C WRITE(N6,106) TEP,TEPP2,(QE(L,2),L=1,NJSW)
106 FORMAT(1X,F6.2,2X,F6.2,3X,10F10.1,/, (18X,10F10.1),/)
235 CONTINUE
TIME=TZERO
C
C INITIAL TIME-STAGE RELATIONSHIP
C COMPUTED
C
IF (ISWCH(1).NE.1) GO TO 236
H(JGW)=A1+A2*SIN(W*T)+A3*SIN(2.*W*T)+A4*SIN(3.*W*T)
1 +A5*COS(W*T)+A6*COS(2.*W*T)+A7*COS(3.*W*T)
236 CONTINUE
C
C CHANNEL CONSTANTS COMPUTED
C
IF(LDD.GT.1) GO TO 2711
DO 280 N=1,NC
IF(NJUNC(N,1).LE.0)GO TO 280
NL=NJUNC(N,1)
NH=NJUNC(N,2)
R(N)=R(N)+(H(NL)+H(NH))/2.
IF(R(N).LE.0.0) R(N)=0.0
A(N)=B(N)*R(N)
IF (WIND.LE.0.0) GO TO 270
FWIND(N)=-WIND*2*COS(WDIR/57.-ATAN((X(NH)-X(NL)),(Y(NH)-Y(NL))))
1 *8.64E-6
270 CONTINUE
AT(N)=A(N)
OAVE(N)=0.
280 CONTINUE
C
C NODAL VOLUMES COMPUTED
C
2711 DO 340 J=1,NJ
VOL(J)=0.
IF(AS(J).EQ.0.) GO TO 340
AREA=0.

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	VOLUME=0.	SWFL251
	DO 300 K=1,10	
	N=NCHAN(J,K)	SWFL253
	IF(N.LE.0) GO TO 300	SWFL254
	AREA=AREA+B(N)*LEN(N)	SWFL255
	VOLUME=VOLUME+B(N)*LEN(N)*(R(N)+RD)	SWFL256
	IF(VOL(J).LE.0.0) VOL(J)=0.0	
300	CONTINUE	SWFL257
	DEPTH=VOLUME/AREA	
	VOL(J)=DEPTH*AS(J)	SWFL259
340	CONTINUE	SWFL260
C		SWFL261
C		START OF PROGRAM CORE, WITH
C		MAJOR HYDRAULIC COMPUTATIONS
C		SWFL262
C		SWFL263
C		SWFL264
C		START OF DAY DO LOOP, OUTER DO
C		LOOP OF 3 NESTED DO LOOPS
C		SWFL265
		SWFL266
		SWFL267
		SWFL268
	DO 1300 NT=1,NTCYC	
C	WRITE(N22) NEXIT,NHCYC,NT,DELT,TZERO,NHPRT,NQPR,T,NJSW	
C	DO 1000 J=1,NQPR	
C	WRITE(N22) (NJUNC(J,K),K=1,2)	
C1000	CONTINUE	
	IF(NT.LT.NQSWRT) GO TO 350	SWFL269
	DO 345 J=1,NPLT	SWFL271
	I=IABS(JPLT(J))	SWFL272
345	HPLT(J)=H(I)	SWFL273
	HOURL=TZERO/3600.	SWFL274
	WRITE(N20) HOURL,(HPLT(J),J=1,NPLT)	SWFL275
	NPTOT=1	SWFL276
	IF(NINREC.GE.NSTEPS.OR.NINREC.EQ.2)GO TO 347	SWFL277
	WRITE(N6,7097)TITEL2	
	WRITE(N6,7093)	
	WRITE (N6,7091) NSTEPS,MJSW,NQUAL,TDELT,TZERO,TAREA	SWFL280
	WRITE(N6,7095)	
	WRITE(N6,7092) (ISW(L),L=1,MJSW)	
	WRITE(N6,7096)	
347	CONTINUE	SWFL284
350	CONTINUE	SWFL285
	LTIME=0	
C		SWFL287
C		START OF QUALITY DO LOOP
C		SWFL288
		SWFL289
	NQCYC=NTIMST	
	DO 1240 NQ=1,NQCYC	SWFL290
	KDAY=KDAY+1	
C		SWFL291
C		INITIALIZATION OF ARRAYS USED
C		FOR HYDRAULIC OUTPUT TO BE USED
C		BY THE SQWAL SUBROUTINE
C		SWFL292
		SWFL293
		SWFL294
		SWFL295
	IF (NT.LT.NQSWRT) GO TO 380	SWFL296
	DO 360 N=1,NC	SWFL297
	VBAR(N)=0.	SWFL298
360	QBAR(N)=0.	SWFL299
	DO 370 J=1,NJ	SWFL300
	HBAR(J)=0.	SWFL301
	QINBAR(J)=0.	SWFL302
	QOUBAR(J)=0.	SWFL303
370	CONTINUE	SWFL304
380	CONTINUE	SWFL305
C		SWFL306

C		START OF HYDRAULIC DO LOOP,	SWFL307
C		INNERMOST DO LOOP OF 3 NESTED	SWFL308
C		DO LOOPS	SWFL309
C			SWFL310
	S12Q=0.0		
	DO 1040 NHH=1,NHCYC		SWFL311
	IF (NT.LE.NQSWRT) GO TO 520		SWFL312
	TIME=TIME+DELT		SWFL313
C			SWFL314
C		PRECIPITATION COMPUTATIONS FOR	SWFL315
C		EACH TIME STEP	SWFL316
	PREC=0.		SWFL317
	390 IF (KRAIN-INRAIN) 395,410,410		SWFL318
	395 IF (TIME-INTIME(KRAIN+1)) 405,400,400		SWFL319
	400 PREC=PREC+RAIN(KRAIN)*(INTIME(KRAIN+1)-TOLD)/(12.*3600.)		SWFL320
	KRAIN=KRAIN+1		SWFL321
	TOLD=INTIME(KRAIN)		SWFL322
	GO TO 390		SWFL323
	405 PREC=(PREC+RAIN(KRAIN)*(TIME-TOLD)/(12.*3600.))/DELT		SWFL324
	TOLD=TIME		SWFL325
	410 CONTINUE		SWFL326
	IF (N21.EQ.0) GO TO 445		SWFL327
	DO 418 L=1,MJSW		SWFL328
	J=ISW(L)		SWFL329
	418 QIN(J)=QINST(J)*DELT		SWFL330
	420 IF (TIME-TT(I2)) 435,425,425		SWFL331
	425 DO 430 L=1,MJSW		SWFL332
	J=ISW(L)		SWFL333
	430 QIN(J)=QIN(J)+QT(L,I1)*(TT(I2)-TTP)		SWFL334
	TTP=TT(I2)		SWFL335
	ITEMP=I2		SWFL336
	I2=I1		SWFL337
	I1=ITEMP		SWFL338
	IF (NINREC-NSTEPS) 431,432,432		SWFL339
	431 READ(N21) TT(I2),(QT(L,I2),L=1,MJSW)		SWFL340
	NINREC=NINREC+1		SWFL341
	WRITE(N6,7094) NINREC,TT(I2),(QT(L,I2),L=1,MJSW)		SWFL342
	GO TO 420		SWFL343
	432 TT(I2)=1000000.		SWFL344
	DO 433 L=1,MJSW		SWFL345
	433 QT(L,I2)=0.		SWFL346
	GO TO 420		SWFL347
	435 DO 440 L=1,MJSW		SWFL348
	J=ISW(L)		SWFL349
	440 QIN(J)=(QIN(J)+QT(L,I1)*(TIME-TTP))/DELT		SWFL350
	TTP=TIME		SWFL351
	445 CONTINUE		SWFL352
	IF (NJSW.EQ.0) GO TO 520		SWFL353
C			SWFL354
C		READ HYDROGRAPH INPUT OR AVERAGES	SWFL355
C		OR INTERPOLATE FOR TIME STEP	SWFL356
C			SWFL357
	IF (TIME.LE.TE) GO TO 480		SWFL358
	TEQ=TE		SWFL359
	DO 460 L=1,NJSW		SWFL360
	460 QE(L,1)=QE(L,2)		SWFL361
			SWFL362
C			SWFL363
C		READ HYDROGRAPHS	SWFL364
C			SWFL365
	IF (ISWCH(3).NE.4) GO TO 469		SWFL366
	IF (TEEM(NNTTM).EQ.TEE) GO TO 480		SWFL367

	NNTTM=NNTTM+1	SWFL368
	READ(N5,462)(DRAIN(LT),LT=1,NGAGE)	SWFL369
462	FORMAT(16F5.2)	SWFL370
	DO 468 JLT=1,NJSW	SWFL371
	SUMGAG=0.	SWFL372
	DO 463 JLK=1,NGAGE	SWFL373
463	SUMGAG=SUMGAG+RCENT(JLT,JLK)*DRAIN(JLK)	SWFL374
	JNUM=JSW(JLT)	SWFL375
468	QE(JLT,2)=EXXT(NNTTM,JLT)+.96*ABS(AS(JNUM))*SUMGAG/(12*86400)	SWFL376
	TE=TEEM(NNTTM)*TCN	SWFL377
	GO TO 470	SWFL378
C		SWFL379
	469 READ (N5,104) TE,(QE(JJ,2),JJ=1,NJSW)	SWFL380
	104 FORMAT (8F10.0)	SWFL381
	470 CONTINUE	SWFL382
	TEP=TE/3600.	SWFL383
	IF(TE.GT.TEPP)GO TO 480	SWFL384
	TEPP2=TEP/24.	SWFL385
C	WRITE(N6,106) TEP,TEPP2,(QE(L,2),L=1,NJSW)	SWFL386
C		SWFL387
C	INTERPOLATE HYDROGRAPH	SWFL388
	DO 500 L=1,NJSW	SWFL391
C		SWFL389
	J=JSW(L)	SWFL392
	SLOPE=(QE(L,2)-QE(L,1))/(TE-TEO)	SWFL393
500	QIN(J)=QINST(J)+QE(L,1)+SLOPE*(TIME-TEO)	SWFL394
520	CONTINUE	SWFL395
480	CONTINUE	SWFL390
C		SWFL396
C	INITIALIZATION	SWFL397
C		SWFL398
	T2=T+DELT2	SWFL399
	T=T+DELT	SWFL400
	DO 525 N=1,NC	SWFL401
	NCLOS(N)=0	SWFL402
	DO 525 M=1,2	SWFL403
525	NJUNC(N,M)=IABS(NJUNC(N,M))	SWFL404
	DO 530 J=1,NJ	SWFL405
	AS(J)=ABS(AS(J))	SWFL406
	DO 530 K=1,10	
530	NCHAN(J,K)=IABS(NCHAN(J,K))	SWFL408
	NTIMS=0	SWFL409
C		SWFL410
C	COMPUTATIONS OF VELOCITIES AT	SWFL411
C	HALF TIME STEP, AND FLOWS AT	SWFL412
C	QUARTER TIME STEP	SWFL413
C		SWFL414
	540 CONTINUE	SWFL415
	NDRY=0	SWFL416
	NTIMS=NTIMS+1	SWFL417
	DO 580 N=1,NC	SWFL418
	IF(NJUNC(N,1).LE.0)GO TO 580	SWFL419
C		SWFL420
C	DRY CHANNEL CHECK (UNDER 0.1 FT)	SWFL421
C		SWFL422
	IF(R(N).GT.0.10) GO TO 560	
	VT(N)=0.0	SWFL424
	Q(N)=0.0	SWFL425
	GO TO 580	SWFL426
560	CONTINUE	SWFL427
	AD=R(N)+RD	
	IF(N.GT.NCGT) AD=R(N)	

	NL=NJUNC(N,1)	SWFL428
	NH=NJUNC(N,2)	SWFL429
	IF(ISWCH(7).NE.1)GO TO 565	SWFL430
	IF(ISWCH(5).EQ.1.AND.N.GT.NCGT)GO TO 565	SWFL431
564	IF(AD.LT.0.5) AK(N)=5.5	
	IF(AD.GE.0.5.AND.AD.LE.5.0) AK(N)=0.39+.4/AD	
	IF(AD.GT.5.0) AK(N)=1.8/AD-0.003*AD	
565	IF(AT(N)-A(N)) 7,7,8	
8	IF(A(N).LE.0.0) A(N)=AT(N)	
7	DELV2=V(N)*(1.-AT(N)/A(N))	SWFL435
	DDD=DELT2*(V(N)*V(N)*B(N)/A(N)-32.1739)	
	DDE=ABS(DDD)	
	IF(DDE.LE.1.0E-20) DDD=0.0	
	IF(N.GT.NCGT) GO TO 394	
	DD1=-FDEP(NH)	
	DDC=-FDEP(NL)	
	AB=DDC	
	IF(DD1.GT.DDC) AB=DD1	
	IF(H(NH).GE.AB.AND.H(NL).GE.AB) GO TO 394	
	DH=0.5*(H(NH)-H(NL))	
	GO TO 393	
394	DH=H(NH)-H(NL)	
393	DELV2=DELV2+DDD*DH/LEN(N)	
	DELV2=DELV2+FWIND(N)/R(N)*DELT2	
	V2=V(N)+DELV2	SWFL438
	TEMP=DELT2*AK(N)/AD**1.33333	SWFL43
	DD1=1./TEMP+2.*ABS(V2)	
	DDE=DD1**2-4.*V2*V2	
	IF(DDE.LT.0.0) DDE=0.0	
	DD2=SQRT(DDE)	
	DELV1=0.5*(DD1-DD2)	
	DELV1=-SIGN(DELV1,V2)	SWFL442
383	VT(N)=V(N)+DELV1+DELV2	SWFL443
	IF(N.GT.NCGT) GO TO 374	
	IF(ABS(VT(N)).GT.10.0) VT(N)=0.05	
374	Q(N)=VT(N)*A(N)	SWFL444
580	CONTINUE	SWFL445
		SWFL446
		SWFL447
		SWFL448
		SWFL449
		SWFL450
		SWFL451
	DO 660 J=1,NJ	
	SUMQ=0.	
	DO 620 K=1,10	
	IF(NCHAN(J,K).LE.0) GO TO 620	SWFL453
	N=NCHAN(J,K)	SWFL454
	IF(J.NE.NJUNC(N,1))GO TO 600	SWFL455
	SUMQ=SUMQ+Q(N)	SWFL456
	GO TO 620	SWFL457
600	SUMQ=SUMQ-Q(N)	SWFL458
620	CONTINUE	SWFL459
	IF(AS(J).LE.0.) GO TO 660	
	C=0.233*H(J)-2.26	
	IF(H(J).LE.10.0) C=0.1	
14	SUMQ=QDU(J)*C-QIN(J)+(EVAP-PREC)*AS(J)+SUMQ	
	IF(ISWCH(1).EQ.2)GO TO 644	SWFL463
	IF(J.EQ.JGW.AND.ISWCH(1).NE.1) GO TO 650	SWFL464
644	HT(J)=H(J)-DELT2*SUMQ/AS(J)	SWFL465
	IF(HT(J)+DEP(J).GT.0.) GO TO 660	SWFL466
	VOL(J)=VOL(J)-SUMQ*DELT2	
	IF(VOL(J).GT.0.0) GO TO 6	
	VOL(J)=0.0	

```

HT(J)=-DEP(J)
DDE=ABS(FDEP(J))
IF(HT(J).LE.DDE) HT(J)=-FDEP(J)
GO TO 16
6 DDE=ABS(FDEP(J))
IF(HT(J).LT.DDE) HT(J)=DDE
DEP(J)=-HT(J)
16 AS(J)=-AS(J)
17 DO 645 K=1,10
NX=NCHAN(J,K)
IF(NX.LE.0) GO TO 645
NCLOS(NX)=1
645 CONTINUE
NDRY=NDRY+1
GO TO 660
650 CONTINUE
DELHH=0.
IF(H(IND).GT.REG(KDAY)) GO TO 6551
DELHH=DELT2/AS(J)*(-SUMQ)
GO TO 656
6551 DO 655 ICT=1,3
BASE=H(JGW)-WEIR2+DELHH/2.0
IF(H(JGW).GE.13.2) BASE=12.5-WEIR2+DELHH/2.0
IF(H(JGW).GE.14.0) BASE=13.0-WEIR2+DELHH/2.0
IF(BASE.LE.0.0) BASE=0.0
DELHH=DELT2/AS(J)*(-SUMQ-WEIR1*BASE**WEIR3)
655 CONTINUE
656 HT(J)=H(J)+DELHH
660 CONTINUE
IF(NDRY.EQ.0) GO TO 675
IF(NTIMS.GT.2) GO TO 675
DO 670 N=1,NC
IF(NJUNC(N,1).LE.0) GO TO 670
IF(NCLOS(N).NE.1) GO TO 670
IF(VOL(J).GT.0.0) GO TO 11
Q(N)=0.
V(N)=0.
11 DO 668 I=1,2
II=NJUNC(N,I)
DO 664 J=1,10
IF(NCHAN(II,J).EQ.N) GO TO 666
664 CONTINUE
GO TO 668
666 NCHAN(II,J)=-N
668 NJUNC(N,I)=-II
670 CONTINUE
GO TO 540
675 CONTINUE

```

SWFL471
SWFL472
SWFL473
SWFL474
SWFL475
SWFL476
SWFL477
SWFL478

SWFL479

SWFL481

SWFL483
SWFL484
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SWFL486
SWFL487
SWFL488

SWFL489
SWFL490

SWFL492

SWFL494
SWFL495
SWFL496
SWFL497
SWFL498
SWFL499
SWFL500
SWFL501
SWFL502

SWFL503
SWFL504
SWFL505

SWFL506
SWFL507
SWFL508
SWFL509
SWFL510
SWFL511
SWFL512
SWFL513
SWFL514
SWFL515

BOUNDARY STAGE CONDITION AT
HALF TIME STEP

COMPUTATION OF CHANNEL CROSS-
SECTIONAL AREAS AT HALF TIME
STEP, FLOWS AT HALF TIME STEP,
AND VELOCITIES AT FULL TIME STEP

	DO 740 N=1,NC	SWFL516
	IF(NJUNC(N,1).LE.0)GO TO 740	SWFL517
	NL=NJUNC(N,1)	SWFL518
	NH=NJUNC(N,2)	SWFL519
	DELH=0.5*(HT(NH)-H(NH)+HT(NL)-H(NL))	SWFL520
	RNT=R(N)+DELH+RD	SWFL521
	IF(N.GT.NCGT) RNT=R(N)+DELH	
	AT(N)=A(N)+B(N)*DELH	SWFL522
		SWFL523
	DRY CHANNEL CHECK (UNDER 0.1 FT)	SWFL524
		SWFL525
	IF(RNT.GT.0.10) GO TO 680	
	V(N)=0.	SWFL527
	Q(N)=0.	SWFL528
	GO TO 700	SWFL529
680	CONTINUE	SWFL530
	IF(ISWCH(7).NE.1)GO TO 685	SWFL531
	IF(ISWCH(5).EQ.1.AND.N.GT.NCGT)GO TO 685	SWFL532
	AD=RNT + RD	
682	IF(AD.LT.0.5) AK(N)=5.5	
	IF(AD.GE.0.5.AND.AD.LE.5.0) AK(N)=0.39+.4/AD	
	IF(AD.GT.5.0) AK(N)=1.8/AD-0.003*AD	
685	IF(A(N)-AT(N)) 9,9,10	
10	IF(AT(N).LE.0.0) AT(N)=A(N)	
9	CONTINUE	
	IF(N.GT.NCGT) GO TO 391	
	DDE=-FDEP(NH)	
	DDC=-FDEP(NL)	
	AB=DDC	
	IF(DDE.GT.DDC) AB=DDE	
	IF(HT(NH).GE.AB.AND.HT(NL).GE.AB) GO TO 391	
	DH=0.5*(HT(NH)-HT(NL))	
	GO TO 392	
391	DH=HT(NH)-HT(NL)	
392	DELV2=2.*VT(N)*(1.-A(N)/AT(N))	
	1+DELT*((VT(N)*VT(N)*B(N)/AT(N))-32.1739)*DH/LEN(N)	SWFL537
	1+FWIND(N)/RNT*DELT	SWFL538
	V2= V(N)+DELV2	SWFL539
	TEMP=DELT*AK(N)/RNT**1.3333333	SWFL540
	AASQRT=(1./TEMP+2.*ABS(V2))*(1./TEMP+2.*ABS(V2))-4.*V2*V2	SWFL541
	IF(AASQRT.LE.1.0E-10)AASQRT=0.0	SWFL542
	DELV1=0.5*((1./TEMP+2.*ABS(V2))-SQRT(AASQRT))	SWFL543
	DELV1=-SIGN(DELV1,V2)	SWFL544
387	V(N)=V(N)+DELV1+DELV2	SWFL545
	IF(N.GT.NCGT) GO TO 385	
	IF(ABS(V(N)).GT.10.0) V(N)=0.05	
385	Q(N)=0.5*(Q(N)+V(N)*AT(N))	SWFL546
700	CONTINUE	SWFL547
		SWFL548
	CHANNEL FLOWS SUMMED	SWFL549
		SWFL550
	IF (NT.LT.NQSWRT) GO TO 720	SWFL551
	QBAR(N)=QBAR(N)+Q(N)	SWFL552
	VBAR(N)=VBAR(N)+V(N)	SWFL553
720	CONTINUE	SWFL554
		SWFL555
	EXCESSIVE VELOCITY CHECK	SWFL556
		SWFL557
	IF(ABS(V(N)).LE.20.0)GO TO 740	SWFL558
	WRITE(N6,108)N,NT,NQ,NHH,R(N),V(N),Q(N),AT(N),A(N),DELV1,DELV2,V2,	SWFL559
	1HT(NH),HT(NL),H(NH),H(NL),AK(N)	SWFL560
108	FORMAT(1H ,7HCHANNEL,15,3X,33HVELOCITY OVER 20 FPS, TIDAL CYCLE,	

	1 I4,12H QUAL CYCLE,I4,13H HYDRO CYCLE,I4,7H DEPTH,E7.2/1H ,	
	28HVELOCITY,E12.2,6H FLOW,E12.2,6H AT =,E12.0,4H A=,E12.0,	
	3 9H DELV1 =,E12.1/1H ,7HDELV2 =,E12.1,6H V2 =,E12.1,1X,	
	48HHT(NH) =,E12.2,1X,8HHT(NL) =,E12.2,7HH(NH) =,E12.2,1X,7HH(NL) =,	
	5 E12.2/1H ,7HAK(N) =,E12.2,10X,8HSWFL 520)	
	CALL EXIT	
740	CONTINUE	SWFL568
C		SWFL569
C	COMPUTATION OF NODAL STAGE AND	SWFL570
C	VOLUME AT FULL TIME STEP	SWFL571
C		SWFL572
	DO 900 J=1,NJ	
	SUMQ=0.	SWFL574
	HN(J)=-DEP(J)	SWFL575
	DDE=-FDEP(J)	
	IF(HN(J).LT.DDE) HN(J)=-FDEP(J)	
	IF(AS(J).LE.0.) GO TO 900	SWFL576
	DO 800 K=1,10	
	IF(NCHAN(J,K).LE.0) GO TO 800	SWFL578
	N=NCHAN(J,K)	SWFL579
	IF(J.NE.NJUNC(N,1))GO TO 780	SWFL580
	SUMQ=SUMQ+Q(N)	SWFL581
	GO TO 800	SWFL582
780	SUMQ=SUMQ-Q(N)	SWFL583
800	CONTINUE	SWFL584
	IF(J.NE.JGW) GO TO 820	SWFL585
	IF(ISWCH(1).EQ.2)GO TO 820	SWFL586
	IF (ISWCH(1).NE.1) GO TO 802	SWFL587
	HN(JGW)=A1+A2*SIN(W*T)+A3*SIN(2.*W*T)+A4*SIN(3.*W*T)	SWFL588
1	+A5*COS(W*T)+A6*COS(2.*W*T)+A7*COS(3.*W*T)	SWFL589
	GO TO 814	SWFL590
802	CONTINUE	SWFL591
	DELHH=DELHH*2.	SWFL592
	IF(H(IND).GT.REG(KDAY)) GO TO 8021	
	DELHH=DELH/AS(J)*(-SUMQ)	
	GO TO 809	
8021	DO 808 ICT=1,3	SWFL593
	BASE=H(JGW)-WEIR2+DELHH/2.0	
	IF(H(JGW).GE.13.2) BASE=12.5-WEIR2+DELHH/2.0	
	IF(H(JGW).GE.14.0) BASE=13.0-WEIR2+DELHH/2.0	
	IF(BASE.LE.0.0) BASE=0.0	
	QQ=WEIR1*BASE**WEIR3	
	DELHH=DELH/AS(J)*(-SUMQ-QQ)	
808	CONTINUE	SWFL595
	S12Q=S12Q+QQ*DELT/86400.0	
809	HN(J)=H(J)+DELHH	SWFL596
814	CONTINUE	SWFL597
	DVOL=(HN(JGW)-H(JGW))*AS(JGW)	SWFL598
	QOU(JGW)=0.	SWFL599
	QIN(JGW)=(DVOL/DELT)+SUMQ	SWFL600
	IF (QIN(JGW).GT.0.) GO TO 815	SWFL601
	QOU(JGW)=-QIN(JGW)	SWFL602
	QIN(JGW)=0.	SWFL603
815	VOL(JGW)=VOL(JGW)+DVOL	SWFL604
	IF(HN(JGW)+DEP(JGW).GT.0.0) GO TO 825	
	IF(VOL(JGW).GT.0.0) GO TO 824	
	VOL(JGW)=0.0	
	HN(JGW)=-DEP(JGW)	
	GO TO 825	
824	IF(HN(JGW).LE.WEIR2) HN(JGW)=WEIR2	SWFL605
	DEP(JGW)=-HN(JGW)	
	DDE=ABS(FDEP(JGW))	

IF(HN(JGW).LT.DDE) HN(JGW)=-FDEP(JGW)
GO TO 825

C
C
C
C

COMPUTATION OF [ORDINARY] NODES
VOLUME AND STAGE

SWFL606
SWFL607
SWFL608
SWFL609

820 C=0.233*H(J)-2.26
IF(H(J).LE.10.0) C=0.1
19 SUMQ=QQU(J)*C-QIN(J)+(EVAP-PREC)*AS(J)+SUMQ
HN(J)=H(J)-DELT*SUMQ/AS(J)
VOL(J)=VOL(J)-DELT*SUMQ
IF(VOL(J).LE.0.0) VOL(J)=0.0

825 CONTINUE
900 CONTINUE

SWFL611
SWFL612
SWFL 613
SWFL614
SWFL615

C
C
C

NODAL VOLUMES AND FLOWS SUMMED

IF (NT.LT.NQSWRT) GO TO 940
DO 920 J=1,NJ
HBAR(J)=HBAR(J)+HN(J)
QINBAR(J)=QINBAR(J)+QIN(J)
QOUBAR(J)=QOUBAR(J)+QQU(J)

920 CONTINUE
940 CONTINUE

SWFL616
SWFL617
SWFL618
SWFL619
SWFL620
SWFL621
SWFL622
SWFL623
SWFL624
SWFL625

C
C
C
C
C

FULL TIME STEP COMPUTATION OF
HYDRAULIC RADIUS AND CHANNEL
CROSS-SECTIONAL AREAS

SWFL626
SWFL627
SWFL628
SWFL629
SWFL630

DO 980 N=1,NC
IF(NJUNC(N,1).EQ.0) GO TO 980
NL=IABS(NJUNC(N,1))
NH=IABS(NJUNC(N,2))
DELH=0.5*(HN(NH)-H(NH)+HN(NL)-H(NL))
R(N)=R(N)+DELH
IF(N.GT.NCGT) GO TO 982
DDE=0.5*(HN(NH)+HN(NL)+FDEP(NH)+FDEP(NL))
IF(R(N).LT.DDE) GO TO 982
R(N)=DDE

982 IF(R(N).LE.0.0) R(N)=0.0
A(N)=A(N)+B(N)*DELH
980 CONTINUE

SWFL632
SWFL633
SWFL634
SWFL635
SWFL636

C
C
C

NODAL STAGE ARRAYS SHIFTED

SWFL637
SWFL638
SWFL639
SWFL640
SWFL641

DO 1020 J=1,NJ
1020 H(J)=HN(J)
IF(NT.LT.NQSWRT) GO TO 1040
IF(NPTOT.NE.NPDEL) GO TO 1030
DO 1025 J=1,NPLT
I=IABS(JPLT(J))
1025 HPLT(J)=H(I)
HOUR=HOUR+DELT/3600.*NPDEL
WRITE(N20) HOUR,(HPLT(J),J=1,NPLT)
NPTOT=0
1030 NPTOT=NPTOT+1

C
C
C
C
C

END OF HYDRAULIC OR INNER DO
LOOP

1040 CONTINUE

SWFL643
SWFL644
SWFL645
SWFL646
SWFL647
SWFL648
SWFL649
SWFL650
SWFL651
SWFL652
SWFL653
SWFL654
SWFL655
SWFL656
SWFL657
SWFL658

C		SWFL659
C		SWFL660
C		SWFL661
	IF (NT.LT.NQSWRT) GO TO 1100	SWFL662
	DO 1060 N=1,NC	SWFL663
	IF(NJUNC(N,1).LE.O) GO TO 1060	SWFL664
	QBAR(N)=QBAR(N)/FLOAT(NHCCYC)	SWFL665
	VBAR(N)=VBAR(N)/FLOAT(NHCCYC)	SWFL666
	QAVE(N)=QAVE(N)+QBAR(N)/FLOAT(NQCCYC)	SWFL667
1060	CONTINUE	SWFL668
	DO 1080 J=1,NJ	SWFL669
	QINBAR(J)=QINBAR(J)/FLOAT(NHCCYC)	SWFL670
	QOUBAR(J)=QOUBAR(J)/FLOAT(NHCCYC)	SWFL671
	HBAR(J)=HBAR(J)/FLOAT(NHCCYC)	SWFL672
	IF(QINBAR(J).EQ.O.) GO TO 1080	SWFL673
	IF(QOUBAR(J).EQ.O.) GO TO 1080	SWFL674
	QINBAR(J)=QINBAR(J)-QOUBAR(J)	SWFL675
	QOUBAR(J)=O.	SWFL676
	IF(QINBAR(J).GT.O.) GO TO 1080	SWFL677
	QOUBAR(J)=-QINBAR(J)	SWFL678
	QINBAR(J)=O.	SWFL679
1080	CONTINUE	SWFL680
C		SWFL681
C		SWFL682
C	WRITE HYDRAULIC INFORMATION FOR	SWFL683
C	USE IN QUALITY PROGRAM	SWFL684
	WRITE(N20) NQ,(QBAR(N),VBAR(N),N=1,NC),	SWFL685
	1 (VOL(J),QINBAR(J),QOUBAR(J),J=1,NJ)	SWFL686
C		SWFL687
C		SWFL688
C	STORE OUTPUT FOR SUBSEQUENT	SWFL689
C	PRINTOUT	SWFL690
	1100 IF (NT.EQ.(NQSWRT-1).AND.NQ.EQ.NQCCYC) GO TO 1120	SWFL691
	GO TO 1180	SWFL692
1120	DO 1140 I = 1,NHPRT	SWFL693
	MJPRT = JPRT(I)	SWFL694
	PRTH(1,I) = H(MJPRT)	SWFL695
1140	CONTINUE	SWFL696
	DO 1160 I = 1,NQPRT	SWFL697
	MCPRT = CPRT(I)	SWFL698
	PRTQ(1,I) = Q(MCPRT)	SWFL699
	PRTV(1,I) = V(MCPRT)	SWFL700
1160	CONTINUE	SWFL701
	GO TO 1240	SWFL702
1180	IF(NT.LT.NQSWRT) GO TO 1240	SWFL703
	LTIME = LTIME + 1	SWFL704
C		SWFL705
C		SWFL706
C	STORE STAGE INFORMATION	SWFL707
	DO 1200 I=1,NHPRT	SWFL708
	MJPRT=JPRT(I)	SWFL709
1200	PRTH(1,I)=H(MJPRT)	
	QNEW(NQ)=S12Q	
	WRITE(N22) LTIME,(PRTH(1,I),I=1,NHPRT)	
	WRITE(61,1) (PRTH(1,I),I=1,NHPRT)	
	1 FORMAT(10E11.3)	
C		SWFL711
C		SWFL712
C	STORE FLOWS AND VELOCITIES	SWFL713
	DO 1220 I=1,NQPRT	SWFL714
	MCPRT=CPRT(I)	SWFL715
	PRTQ(1,I)=Q(MCPRT)	

1220	PRTV(1,I)=V(MCPRT)	
	WRITE(N22) LTIME,(PRTV(1,I),I=1,NQPRT)	
C	WRITE(61,1) (PRTV(1,I),I=1,NQPRT)	
	WRITE(N22) LTIME,(PRTV(1,I),I=1,NQPRT)	
C		SWFL718
C	END OF QUALITY DO LOOP	SWFL719
C		SWFL720
1240	CONTINUE	SWFL721
	IF (ISWCH(1).NE.1) GO TO 1280	SWFL722
	IF (NT.NE.NQSWRT) GO TO 1280	SWFL723
1280	CONTINUE	SWFL725
C		SWFL726
C		SWFL727
C	SUBROUTINE PRTOUT CALLED FOR	SWFL728
C	HYDRAULIC INFORMATION PRINTOUT	SWFL729
C	FOR A ONE DAY CYCLE	SWFL730
		SWFL731
	IF (NT.LT.NQSWRT) GO TO 1300	
C	WRITE(N22) (CPRT(I),I=1,NQPRT)	
C	WRITE(N22) (JPRT(I),I=1,NHPRT)	
C	WRITE(N22) (JSW(I),I=1,NJSW)	
C		SWFL733
C	END OF SUBROUTINE SWFLOW	SWFL734
C		SWFL735
1300	CONTINUE	SWFL736
	WRITE(61,232) (QNEW(I),I=1,NTIMST)	
	IF (JMD-NMON) 2,41,41	
2	JMD=JMD+1	
	DELT=3600.0	
	TZERO=0.0	
	DO 5 J=1,NJ	
	H(J)=PRTH(1,J)	
	DEP(J)=-H(J)	
	IF (AS(J).LT.0.0) AS(J)=-AS(J)	
	QIN(J)=DOIN(J)	
	QOU(J)=DOOU(J)	
5	CONTINUE	
	READ(N5,232) EVAP,WIND,WDIR	
	GO TO 50	
41	IF (IYEAR - ICONT) 47,48,48	
47	IYEAR=IYEAR+1	
	JMD=1	
	GO TO 501	
48	ENDFILE N22	
	REWIND 22	
	DUMMY=0.0	
	TMAX=10000.0	
C	WRITE(N20) TMAX,(DUMMY,J=1,NPLT)	
	END FILE N20	
	REWIND 20	
	MCOUNT = 0	SWFL740
1340	READ(N5,110) IFINAL,ICARD	
110	FORMAT (2A4)	
	IF (IFINAL.EQ.IENDER(1)) GO TO 1360	
	MCOUNT = MCOUNT + 1	SWFL744
	IF (MCOUNT.GT.30) GO TO 1380	SWFL745
	GO TO 1340	SWFL746
1360	IF (ICARD.EQ.IENDER(2)) GO TO 1400	
	MCOUNT = MCOUNT + 1	SWFL748
	IF (MCOUNT.GT.30) GO TO 1380	SWFL749
	GO TO 1340	SWFL750
1380	WRITE (N6,112)	SWFL751
112	FORMAT (62H0QUALITY PROGRAM HAS READ MORE THAN 30 CARDS AFTER COMPSWFL752	

PROGRAM LISTING OF INDATA

1
2
3
4
5
6
7
8


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*****
**
**   DATE= 12/05/78   TIME= 21/41/59
**
**                               SOUTH FLORIDA WATER MANAGEMENT DISTRICT
**
**                               SYST= 33   MACH= E
**
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**   **   **   **   **
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JOB,0075-203,U001,10,,,
FORTRAN,X

U001

FORTRAN DIAGNOSTIC RESULTS FOR U001

NO ERRORS

LOAD,56
RUN

\$JOB,8636-304,LIN,5
 \$RONL,854/6201
 \$RRAT,854/6201
 \$FET,LIN,INDATA,960
 \$RELEASE,ALL
 \$ALLOCATE,29,999999
 \$OPEN,40
 \$COSY

INDATA DECK/ I=60,H=54
 PROGRAM INDATA

C		INDA	1
C		INDA	2
C		INDA	3
C	INPUT DATA	INDA	4
C	HYDRODYNAMICS PROGRAM	INDA	5
C	SPECIFICATION STATEMENTS	INDA	6
C		INDA	7
C		INDA	8
C	CONTROL	INDA	9
C		INDA	10
C		INDA	54
C	TYPE DESIGNATIONS	INDA	55
C		INDA	56
C	INTEGER CPRT	INDA	58
C	REAL LEN,INTIME	INDA	59
C	COMMON N5,N5,ISWCH(10),EN(4),AVGH(99),IICOL(10),T(5),NX(5)		
C		INDA	14
C	GENERAL	INDA	15
C		INDA	16
C	COMMON IALHA(15),NJ,NC		
C		INDA	20
C	JUNCTIONS	INDA	21
C		INDA	22
C	COMMON H(35),NJUNC(99,2),IPOINT(35,12),AS(35),X(35),Y(35),DEP(35)		
C	1, COF(35),QIN(35),QOU(35),QINST(35)		
C		INDA	27
C	CHANNELS	INDA	28
C		INDA	29
C	COMMON LEN(99),NCHAN(99,12),B(99),R(99),A(99),AK(99),V(99)		
C	1, NUMCH(99),NTEMP(12)		
C		INDA	34
C	PRINTOUT AND PLOTTING	INDA	35
C		INDA	36
C	COMMON JPRT(99),CPRT(99),JPLT(99),TT(99)		
C		INDA	40
C		INDA	41
C	STAGE-TIME COEFFICIENTS	INDA	42
C		INDA	43
C		INDA	46
C	STORMWATER	INDA	47
C		INDA	48
C	COMMON ITIL(15),RAIN(50),INTIME(50)		
C	DATA (YES=4H YES),(BLANK=4H)		
C		INDA	60
C	*** OPTION SWITCH, ISWCH(I) ***	INDA	61
C		INDA	62
C	** ISWCH(1) **	INDA	63
C	IF 0, INFLUENCED BY DOWNSTREAM	INDA	63A
C	HEAD RELATIONSHIP (DAM)	INDA	63B
C	IF 1, WILL CALL TIDAL	INDA	64
C	COEFFICIENTS PROGRAM	INDA	65
C	IF 2, SPECIFIED OUTFLOW	INDA	66

C	** ISWCH(2) **	INDA 67
C	IF 0, PRINT INPUT CHANNEL AND	INDA 67A
C	JUNCTION DATA	INDA 67B
C	IF 1, SUPPRESSES CHANNEL AND	INDA 68
C	NODAL INFORMATION PRINT	INDA 69
C	** ISWCH(3) **	INDA 70
C	IF 4, QE(NJSW,2) IS COMPUTED	INDA 71
C	** ISWCH(4) **	INDA 71A
C	IF 1, TRIANGLES ARE USED IN CARD	INDA 71B
C	GROUP 15 AND DIFFERENT MANNING	INDA 71C
C	COEFFICIENTS ARE DESIRED FOR	INDA 71D
C	EACH LEG OF THE TRIANGLE	INDA 71E
C	** ISWCH(5) **	INDA 72
C	IF 1, PARALLEL CHANNELS ARE USED	INDA 73
C	** ISWCH(6) **	INDA 74
C	IF 0, AND SUBROUTINE TRIAN IS	INDA 75
C	USED JUNCTION SURFACE AREA	INDA 76
C	MUST BE LEFT OUT OF INPUT DATA	INDA 77
C	IF 2, AND SUBROUTINE TRIAN IS	INDA 78
C	USED JUNCTION SURFACE AREA	INDA 79
C	MUST BE FURNISHED	INDA 80
C	** ISWCH(7) **	INDA 81
C	IF 1, AK(N) IS COMPUTED IN SWFLOW	INDA 82
C	** ISWCH(8) **	INDA 82A
C	NOT USED	INDA 82B
C	** ISWCH(9) **	INDA 82C
C	NOT USED	INDA 82D
C	** ISWCH(10) **	INDA 83
C	IF ISWCH(5) = 1, THEN CHANNEL	INDA 84
C	NUMBERS GREATER THAN THIS	INDA 85
C	NUMBER (ISWCH(10)) ARE PARALLEL	INDA 86
C	TO OTHER LOWER NUMBERED CHANNELS	INDA 87
C		INDA 88
C	STEP ONE	INDA 89
C	INITIALIZATION	INDA 90
C		INDA 91
C	N5=60	INDA 92
C	N6=61	INDA 93
C	N20=20	
C	DO 50 I=1,35	
C	DEP(I)=0.0	
C	AS(I)=0.0	
C	QIN(I)=0.0	
C	QOU(I)=0.0	
C	50 CONTINUE	
C	DO 60 I=1,12	
C	NTEMP(I)=0	
C	DO 60 J=1,35	
C	NCHAN(J,I)=0	
C	IPOINT(J,I)=0	
C	60 CONTINUE	
C	DO 70 I=1,99	
C	70 B(I)=0.0	
C		
C	REWIND N20	
C		
C	N20 ASSIGNED IN RECEIV	INDA 94
C		INDA 95
C		INDA 96
C	STEP TWO	INDA 97
C	TITLES, GENERAL CONTROL DATA,	INDA 98
C	AND JUNCTION AND CHANNEL INFOR-	INDA 99
C	MATION	INDA100
C		INDA101
C		INDA102

C		READ TYPE A CARDS	INDA103
C		(FIRST TWO CARDS CONTAIN HEAD-	INDA104
C		INGS FOR HYDRODYNAMICS, SECOND	INDA105
C		TWO CARDS CONTAIN HEADINGS	INDA106
C		FOR IDENTIFICATION OF STORMWATER	INDA107
C		INFORMATION)	INDA108
C			INDA109
	READ(N5,100) IALHA		
	READ(N5,100) ITIL		
100	FORMAT(15A4)		INDA112
C		SWITCH INFORMATION	INDA113
C			INDA114
	READ (N5,104) (ISWCH(I),I=1,10)		INDA115
104	FORMAT (10I5)		INDA116
	DO 11 I=1,10		INDA117
	11 IICOL(I)=I		INDA118
	WRITE(N6,6)IICOL,ISWCH		
6	FORMAT(1H1,55X,16H SWITCH SETTINGS//1H,14H SWITCH NUMBER,10I10/		
	1 1H,14H SWITCH SETTING,10I10)		
	WRITE(N6,102) IALHA		
102	FORMAT(1H1,15A4,20X,36HC ^ S FLORIDA FLOOD CONTROL DISTRICT/1H,80		
	1X,22HW. PALM BEACH, FLORIDA/1H,80X,29HRECEIVING WATER HYDRODYNAMI		
	2CS//)		
C			INDA125
C		READ TYPE B CARDS	INDA126
C			INDA127
C		READ TYPE C CARDS	INDA128
C		CONTROL INFORMATION	INDA129
C			INDA130
	READ (N5,106) NTCYC,PERIOD,QINT,DELT,TZERO,NHPRT,NQRT,NPLT,EVAP		INDA131
	1,WIND,WDIR,NQSWRT,NJSW,INRAIN,JGW		INDA132
106	FORMAT (15,4F5.0,3I5,3F5.0,4I5)		INDA133
	NCGT=ISWCH(10)		INDA134
	TTZERO=TZERO*3600.		INDA135
	IPERID = PERIOD + 0.1		INDA136
	IQINT=QINT*3600.+0.1		INDA137
	IDELT = DELT + 0.1		INDA138
	NQCYC=(IPERID *3600)/IQINT		INDA139
	NHCYC = IQINT/IDELT		INDA140
	NINT = (IPERID*3600)/IDELT		INDA141
	NPDEL = (NINT+50)/100		INDA142
C			INDA143
C		READ TYPE D CARDS	INDA144
C		PRECIPITATION IS READ AT THIS	INDA145
C		POINT, RATE IS INCHES PER HOUR,	INDA146
C		TIME IS READ IN MINUTES FROM	INDA147
C		START OF STORM	INDA148
C			INDA149
	DD 210 N=1,50		INDA151
	RAIN(N)=0.0		INDA152
	INTIME(N)=0.0		INDA153
210	CONTINUE		INDA154
	IF (INRAIN.EQ.0) GO TO 215		INDA155
	READ(N5,110){RAIN(I),INTIME(I),I=1,INRAIN)		INDA156
110	FORMAT (8F10.0)		INDA157
215	CONTINUE		INDA158
	DELTQ=DELT*FLOAT(NHCYC)		INDA159
	WRITE(N6,112) NTCYC		INDA160
112	FORMAT (15HODAYS SIMULATED,I4)		INDA161
	WRITE(N6,114) NQCYC		INDA162
114	FORMAT (29HOWATER QUALITY CYCLES PER DAY,I4)		INDA163
	WRITE (N6,116) NHCYC		

116	FORMAT (43HOINTEGRATION CYCLES PER WATER QUALITY CYCLE,I4)	INDA164
	WRITE(N6,118) DELT	
118	FORMAT (30HOLENGTH OF INTEGRATION STEP IS,F6.0,8H SECONDS)	INDA166
	WRITE(N6,120) IZERO	
120	FORMAT (13HOINITIAL TIME,F6.2,6H HOURS)	INDA168
	WRITE(N6,122)EVAP	INDA169
122	FORMAT (18HOEVAPORATION RATE,F5.1,17H INCHES PER MONTH)	INDA170
	WRITE(N6,124)WIND,WDIR	INDA171
124	FORMAT (15HOWIND VELOCITY,F5.0,22H MPH WIND DIRECTION,F5.0,19H DE	INDA172
	IGREES FROM NORTH)	INDA173
	IF (ISWCH(1),NE.1) GO TO 216	INDA174
	WRITE (N6,126)	INDA175
126	FORMAT (16HOESTURIAL SYSTEM)	INDA176
	GO TO 218	INDA177
216	CONTINUE	INDA178
	WRITE (N6,127)	INDA179
127	FORMAT (19HOSTREAM/LAKE SYSTEM)	INDA180
218	CONTINUE	INDA181
	WRITE (N6,128) NOSWRT	INDA182
128	FORMAT (26HOWRITE CYCLE STARTS AT THE,I4,11H TIME CYCLE//)	INDA183
	IF (INRAIN.LE.0) GO TO 225	INDA184
	WRITE (N6,130)	INDA185
130	FORMAT (75HORAIN IN INCHES PER HOUR, AND TIME IN MINUTES, MEASURED	INDA186
	1FROM START OF STORM//)	INDA187
	WRITE (N6,131)	INDA188
131	FORMAT (15X,8H IN./HR.,2X,8H MINUTES,4X,8H IN./HR.,2X,8H MINUTES,	INDA189
	14X,8H IN./HR.,2X,8H MINUTES,4X,8H IN./HR.,2X,8H MINUTES,4X,8H IN./	INDA190
	2HR.,2X,8H MINUTES//)	INDA191
	DO 220 I=1,50,5	
	L =MINO(I + 4,50)	
	WRITE (N6,132) I, L, (RAIN(J),INTIME(J), J=I,L)	INDA194
132	FORMAT(I4,4H TO ,I3,10F11.3)	
	220 CONTINUE	INDA196
	DO 222 I=1,50	
	222 INTIME(I)=INTIME(I)*60.	INDA198
	GO TO 230	INDA199
	225 CONTINUE	INDA200
	WRITE (N6,133)	INDA201
133	FORMAT (23HONO PRECIPITATION INPUT)	INDA202
230	CONTINUE	INDA203
C		INDA204
C		INDA205
C	READ TYPE E CARDS	INDA206
C	JUNCTION NUMBERS FOR DETAILED	INDA207
C	PRINTOUT	INDA208
	READ(N5,134)(JPRT(I),I=1,NHPRT)	INDA209
134	FORMAT(8I10)	INDA210
	WRITE(N6,136)NHPRT,(JPRT(I),I=1,NHPRT)	INDA211
136	FORMAT (32HOPRINTED OUTPUT AT THE FOLLOWING,I3,10H JUNCTIONS, //	INDA212
	1 (10X,16I6))	INDA213
C		INDA214
C		INDA215
C	READ TYPE F CARDS	INDA216
C	CHANNEL NUMBERS FOR DETAILED	INDA217
C	PRINTOUT	INDA218
	READ(N5,134)(CPRT(I),I=1,NOPRT)	INDA219
	WRITE(N6,138)NOPRT,(CPRT(I),I=1,NOPRT)	INDA220
138	FORMAT(//1H0,32HPRINTED OUTPUT FOR THE FOLLOWING,I3,9H CHANNELS,	
	1//((10X,8I10))	
C		INDA223
C		INDA224
C	READ TYPE G CARDS	INDA225
	READ THE JUNCTION NUMBERS IF	

C		PLOTS ARE REQUESTED, OTHERWISE	INDA226
C		SKIP THIS READ	INDA227
C			INDA228
	IF (NPLT,NE.0) READ(N5,134) (JPLT(N),N=1,NPLT)		INDA229
C			INDA230
C		TIDAL OPTION AT THIS POINT	INDA231
C			INDA232
	560 CONTINUE		INDA240
	142 FORMAT (8F10.0)		INDA237
	IF (ISWCH(1).EQ.2) GO TO 580		INDA241
	READ (N5,142) A1,A2,A3		INDA242
	A4=0.		INDA243
	A5=0.		INDA244
	A6=0.		INDA245
	A7=0.		INDA246
	WRITE(N6,162)JGW,A1,A2,A3		INDA247
162	FORMAT(//1H,38H WEIR BOUNDARY CONDITION AT JUNCTION,I5//1H,		
	118H WEIR1 = A1 =,F12.2,4X,14H WEIR2 = A2,F12.2,4X,		
	2 11H WEIR3 =,F12.2//)		
	580 CONTINUE		INDA251
	NJ=0		INDA252
C			INDA253
C		READ CARDS FOR	INDA254
C		NODAL INFORMATION	INDA255
C			INDA256
	DO 620 I=1,35		
	READ(N5,166) J,HEAD,SURF,QF1,QF2,DT,CF,X1,Y1		INDA258
166	FORMAT(I5,F5.0,F10.0,2F5.0,2F10.0,20X,2F5.0)		
	IF(J.GT.30) GO TO 640		
	IF(J.GT.NJ)NJ=J		INDA261
	H(J)=HEAD		INDA262
	AS(J)=SURF*10.0**6		INDA 263
	QIN(J)=QF1		INDA264
	QINST(J)=QF1		INDA265
	QOU(J)=QF2		INDA266
	X(J)=X1*10.0**3		INDA 267
	Y(J)=Y1*10.0**3		INDA 268
	DEP(J)=DT		INDA269
	COF(J)=CF		INDA270
620	CONTINUE		INDA271
640	CONTINUE		INDA272
	NC=0		INDA273
C			INDA274
C		READ CARDS FOR	INDA275
C		CHANNEL INFORMATION	INDA276
C			INDA277
	DO 660 I=1,80		
	READ(N5,172)N,(NTEMP(K),K=1,4),ALEN,WIDTH,RAD,COEF,VEL		INDA279
172	FORMAT(5I5,5F10.0)		INDA280
	IF(N.GT. 80) GO TO 670		INDA 281
	IF(NTEMP(3).NE.0) GO TO 655		INDA282
	NC=NC+1		INDA283
	N=NC		INDA284
	NNTEM1=NTEMP(1)		INDA285
	NNTEM2=NTEMP(2)		INDA286
	R(N)=(DEP(NNTEM1)+DEP(NNTEM2))/2.		INDA287
	IF (ISWCH(5).EQ.1.AND.N.GT.NCGT)R(N)=PAD		INDA288
	A(N)=R(N)*WIDTH		INDA289
	AVGH(N)=(H(NNTEM1)+H(NNTEM2))/2.		INDA290
	LEN(N)=ALEN		INDA291
	B(N)=WIDTH		INDA292
	AK(N)=COEF		INDA293

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V(N)=VEL
NJUNC(N,1)=MINO(NTEMP(1),NTEMP(2))
NJUNC(N,2)=MAXO(NTEMP(1),NTEMP(2))
K=NJUNC(N,1)
DO 643 J=1,12
IF(ISWCH(5).EQ.1)GO TO 642
IF(IPOINT(K,J).EQ.NJUNC(N,2))GO TO 648
642 IF(IPOINT(K,J).EQ.0) GO TO 646
643 CONTINUE
646 IPOINT(K,J)=NJUNC(N,2)
NCHAN(K,J)=NC
GO TO 660
648 NC=NC-1
M=NCHAN(K,J)
NITEM1=NTEMP(1)
NITEM2=NTEMP(2)
R(M)=(DEP(NITEM1)+DEP(NITEM2))/2.
A(M)=R(M)*WIDTH
AVGH(M)=(H(NITEM1)+H(NITEM2))/2.
LEN(M)=ALEN
B(M)=B(M)+WIDTH
AK(M)=COEF
V(M)=VEL
GO TO 660
655 IF(ISWCH(4).EQ.0)GO TO 657
EN(1)=ALEN
EN(2)=WIDTH
EN(3)=RAD
EN(4)=COEF
657 CALL TRIAN(NTEMP(1),NTEMP(2),NTEMP(3),NTEMP(4))
660 CONTINUE
670 CONTINUE
IF (ISWCH(2).EQ.1) GO TO 674
LDELT=DELT
WRITE(N6,170) LDELT
170 FORMAT(129H1CHANNEL LENGTH WIDTH AREA MANNING VELOCIT
1Y HYD RADIUS JUNCTIONS AT ENDS MAX TIME STEP EXC
2EDED BY/1H , 123H
3 NUMBER (FT) (FT) (SQ FT) COEF. (FPS) (FT) INDA332
4 (SEC) STEP OF, I6/)
674 CONTINUE
DO 695 N=1,NC
IF (AK(N).LE.0.0) AK(N)=0.018
IF(B(N).GT.0.) GO TO 683
K=NJUNC(N,1)
NJUNC(N,1)=0
IDEL=0
DO 682 J=1,12
IF(IPOINT(K,J).EQ.0) GO TO 682
IF(IPOINT(K,J).NE.NJUNC(N,2)) GO TO 681
WRITE(N6,168) N,K,NJUNC(N,2)
168 FORMAT (8H CHANNEL,I4,8H JOINING,I4,4H AND,I4,38H DELETED DUE TO ZINDA345
1ERO OR NEGATIVE WIDTH)
NCHAN(K,J)=0
IPOINT(K,J)=0
NJUNC(N,2)=0
GO TO 695
681 CONTINUE
682 CONTINUE
683 CONTINUE
K=NJUNC(N,2)
DO 684 J=1,12

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IF (ISWCH(5).EQ.1.AND.N.GT.NCGT)GO TO 6683
INDA356
IF (IPOINT(K,J).EQ.NJUNC(N,1)) GO TO 687
INDA357
6683 IF (IPOINT(K,J).EQ.0) GO TO 685
INDA358
684 CONTINUE
INDA359
685 CONTINUE
INDA360
IPOINT(K,J)=NJUNC(N,1)
INDA361
NCHAN(K,J)=N
INDA362
687 CONTINUE
INDA363
NUMCH(N)=NJUNC(N,2)+NJUNC(N,1)*1000
INDA364
DO 688 J=1,NQPR
INDA365
IF (CPRT(J).NE.NUMCH(N)) GO TO 688
INDA366
CPRT(J)=N
INDA367
GO TO 690
INDA368
688 CONTINUE
INDA369
690 CONTINUE
INDA370
NLL=NJUNC(N,1)
INDA371
NHH=NJUNC(N,2)
INDA372
RRR=R(N)+(H(NLL)+H(NHH))/2.
INDA373
TF=LEN(N)/SQRT(32.2*(RRR+2.))
INDA374
XMK=BLANK
INDA375
IF (TF.LT.DELT) XMK=YES
INDA376
IF (ISWCH(2).EQ.1) GO TO 695
INDA377
IF (AVGH(N).LT.0.0)GO TO 700
INDA378
RRRR=AVGH(N)+R(N)
INDA379
IF (RRRR.LE.0.0) RRRR=0.0
INDA380
GO TO 710
INDA381
700 RRRR=ABS(AVGH(N)+R(N))
INDA381
710 AAREA=RRRR*B(N)
INDA382
WRITE(N6,174) N,LEN(N),B(N),AAREA,AK(N),V(N),RRRR,(NJUNC(N,K),
INDA383
1 K=1,2),TF,XMK
INDA384
174 FORMAT(I5,F11.0,F10.2, F10.0, F9.3,F10.2,F11.1,I19,I6,F19.0,8X, I
INDA386
1A4)
INDA387
695 CONTINUE
INDA387
IF (ISWCH(2).EQ.1) GO TO 698
INDA388
WRITE(N6,182)
INDA389
182 FORMAT(129H1JUNCTION INITIAL HEAD DEPTH SURFACE AREA INP
INDA390
1UT OUTPUT CHANNELS ENTERING JUNCTION COO
INDA391
2RDINATES)
INDA392
WRITE(N6,8661)
INDA393
8661 FORMAT(1H,68H NUMBER (FT) (FT) (10**6 SQ FT) (
INDA394
1CFS) (CFS),53X,1HX,5X,1HY/)
INDA395
WRITE(N20) NTCYC,PERIOD,DELTA,TZERO,NHPRT,NQPR,NPLT,EVAP,WIND
INDA396
1, WDIR,NQSWRT,NJSW,INRAIN,JGW
INDA397
WRITE(N20) NOCYC,NHCYC,NJ,NC,A1,A2,A3,A4,A5,A6,A7,NPDEL
INDA398
ATOT=0.
INDA399
DO 696 J=1,NJ
INDA400
AS(J)=AS(J)/10.0**6
INDA401
X(J)=X(J)/10.0**3
INDA402
Y(J)=Y(J)/10.0**3
INDA403
ATOT=ATOT+AS(J)
INDA404
WRITE(N6,184)J,H(J),DEP(J),AS(J),QIN(J),QOU(J),(NCHAN(J,K),K=1,12)
INDA405
1,X(J),Y(J)
INDA406
184 FORMAT(I6,F13.2,F12.1, F15.2, 2F10.0,I5,11I4, F8.1,F6.1)
INDA407
X(J)=X(J)*10.0**3
INDA408
Y(J)=Y(J)*10.0**3
INDA409
WRITE(N20)J,H(J),DEP(J),AS(J),QIN(J),QOU(J),(NCHAN(J,K),K=1,12)
INDA410
1, X(J),Y(J)
INDA411
696 CONTINUE
INDA412
ATOTM=ATOT/27878400.*10.0**6
INDA413
WRITE(N6,190) ATOT,ATOTM
INDA414
190 FORMAT(/1H,26HTOTAL AREA FOR THE SYSTEM , F10.2,
INDA415
1 22H * 10**6 SQ FT OR , F9.2,10H SQ MILES/)

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699	CONTINUE		INDA408
	WRITE(N6,192) ITIL		
192	FORMAT(1H0,15A4)		
C			
C		STORE SYSTEM DATA ON QUALITY	INDA411
C		OUTPUT TAPE	INDA412
C			INDA413
			INDA414
	DO 6950 J=1,NC		
	WRITE(N20)LEN(J),B(J),R(J),A(J),AK(J),V(J),(NJUNC(J,K),K=1,2)		
6950	CONTINUE		
	DO 1920 J=1,NJ		
	WRITE(N20) QINST(J),(IPOINT(J,K),K=1,10)		
1920	CONTINUE		
	WRITE(N20)(CPRT(I),I=1,NQPR)		
	WRITE(N20)(JPRT(I),I=1,NHPRT)		
	WRITE(N20)(JPLT(I),I=1,NPLT)		
	WRITE(N20) IALHA,ITIL,(NTEMP(I),I=1,4)		
	WRITE(N20)(ISWCH(I),I=1,10)		
	WRITE(N20)(RAIN(I),INTIME(I),I=1,50)		
	END FILE N20		
	CALL EXIT		
	END		
	SUBROUTINE TRIAN(II,JJ,KK,LL)		INDA418
C			TRIA 1
C		SUBROUTINE TRIAN	TRIA 2
C		HYDRODYNAMICS PROGRAM	TRIA 3
C		SPECIFICATION STATEMENTS	TRIA 4
C			TRIA 5
C			TRIA 55
C		TYPE DESIGNATIONS	TRIA 56
C			TRIA 57
	INTEGER CPRT		TRIA 58
	REAL LEN		TRIA 59
C			TRIA 6
C		CONTROL	TRIA 7
C			TRIA 8
	COMMON N5,N6,ISWCH(10),EN(4),AVGH(99),IICOL(10),T(5),NX(5)		
C			TRIA 11
C		GENERAL	TRIA 12
C			TRIA 13
	COMMON IALHA(15),NJ,NC		
C			TRIA 17
C		JUNCTIONS	TRIA 18
C			TRIA 19
	COMMON H(35),NJUNC(99,2),IPOINT(35,12),AS(35),X(35),Y(35),DEP(35)		
	1, CDF(35),QIN(35),QOU(35),QINST(35)		
C			TRIA 24
C		CHANNELS	TRIA 25
			TRIA 26
	COMMON LEN(99),NCHAN(99,12),B(99),R(99),A(99),AK(99),V(99)		
	1, NUMCH(99),NTEMP(12)		
C			TRIA 31
C		PRINTOUT AND PLOTTING	TRIA 32
C			TRIA 33
	COMMON JPRT(99),CPRT(99),JPLT(99),TT(99)		
C			TRIA 37
C		STAGE-TIME COEFFICIENTS	TRIA 38
C			TRIA 39
C			TRIA 42
C		STORMWATER	TRIA 43
C			TRIA 44
	COMMON ITIL(15),RAIN(50),INTIME(50)		
C			TRIA 51

C		TRIA 52
C		TRIA 54
	IF(II.NE.0) GO TO 300	TRIA 60
C		TRIA 61
C		TRIA 62
	DO 250 I=1,NJ	TRIA 63
	DO 250 J=1,12	TRIA 64
	IPOINT(I,J)=0	TRIA 65
	NCHAN(I,J)=0	TRIA 66
250	CONTINUE	TRIA 67
	RETURN	TRIA 68
C		TRIA 69
C		TRIA 70
C		TRIA 71
300	NX(1)=II	TRIA 73
	NX(2)=JJ	TRIA 74
	NX(3)=KK	TRIA 75
	NX(4)=II	TRIA 76
	NX(5)=JJ	TRIA 77
	T(1) = (X(JJ) - X(KK))**2 + (Y(JJ) - Y(KK))**2	TRIA 78
	T(2) = (X(KK) - X(II))**2 + (Y(KK) - Y(II))**2	TRIA 79
	T(3) = (X(II) - X(JJ))**2 + (Y(II) - Y(JJ))**2	TRIA 80
	T(4)=T(1)	TRIA 81
	T(5)=T(2)	TRIA 82
C		TRIA 83
C		TRIA 84
C		TRIA 85
	DO ALL THREE SIDES	TRIA 86
	NB=2	TRIA 87
	IF(LL.EQ.0) NB=1	TRIA 88
	DO 600 N=1,3,NB	TRIA 89
C		TRIA 90
C		TRIA 91
C		TRIA 92
	I=MINO(NX(N+1),NX(N+2))	TRIA 93
	J=MAXO(NX(N+1),NX(N+2))	TRIA 94
	DO 350 K=1,12	TRIA 95
	IF(IPOINT(I,K).EQ.J) GO TO 370	TRIA 96
	IF(IPOINT(I,K).EQ.0) GO TO 360	TRIA 97
350	CONTINUE	TRIA 98
360	IPOINT(I,K)=J	TRIA 99
	NC=NC+1	TRIA100
	NCHAN(I,K)=NC	TRIA101
370	M=NCHAN(I,K)	TRIA102
C		TRIA103
C		TRIA104
C		TRIA105
	NJUNC(M,1)=I	TRIA106
	NJUNC(M,2)=J	TRIA107
	SUB=T(N+1)+T(N+2)-T(N)	TRIA108
	IF(SUB.LT.1) SUB=1	TRIA109
	G=SQRT(T(N))/2.	TRIA110
	LEN(M)=2.*G	TRIA111
	C=G/SQRT(4.*T(N+2)*T(N+1)-SUB**2)*SUB	TRIA112
	G=G/2.*C	TRIA113
	WRITE(61,2) I,K,IPOINT(I,K),LEN(M),B(M),H(I),H(J)	
2	FORMAT(1X,3I5,4F20.3/)	
	IF(ISWCH(6).EQ.2)GO TO 390	TRIA114
	AS(I)=AS(I)+G	TRIA115
	AS(J)=AS(J)+G	TRIA116
390	CONTINUE	TRIA117
	IF(C.LE.0.) WRITE(N6,102) M,C	

102	FORMAT(1H ,26HNEGATIVE WIDTH CHANNEL NO.,I5,9H WIDTH =,E12.4)	
	B(M)=B(M)+C	TRIA120
	R(M)=(DEP(I)+DEP(J))/2.	TRIA121
	AVGH(M)=(H(I)+H(J))/2.	TRIA122
	A(M)=B(M)*R(M)	TRIA123
	IF(ISWCH(4).NE.1)GO TO 500	TRIA124
	IF(I.EQ.II.AND.J.EQ.JJ.OR.I.EQ.JJ.AND.J.EQ.II)GO TO 400	TRIA125
	IF(I.EQ.JJ.AND.J.EQ.KK.OR.I.EQ.KK.AND.J.EQ.JJ)GO TO 410	TRIA126
	AK(M)=EN(3)	TRIA127
	GO TO 580	TRIA128
400	AK(M)=EN(1)	TRIA129
	GO TO 580	TRIA130
410	AK(M)=EN(2)	TRIA131
	GO TO 580	TRIA132
500	AK(M)=(COF(I)+COF(J))/2.	TRIA133
580	V(M)=0.0	TRIA134
600	CONTINUE	TRIA135
	IF(LL.EQ.0) RETURN	TRIA136
	DO 750 NN=3,4	TRIA137
	I=MINO(NX(NN),LL)	TRIA138
	J=MAXO(NX(NN),LL)	TRIA139
	DO 620 K=1,12	TRIA140
	IF(IPOINT(I,K).EQ.J) GO TO 640	TRIA141
	IF(IPOINT(I,K).EQ.0) GO TO 630	TRIA142
620	CONTINUE	TRIA143
630	IPOINT(I,K)=J	TRIA144
	NC=NC+1	TRIA145
	NCHAN(I,K)=NC	TRIA146
640	M=NCHAN(I,K)	TRIA147
	NJUNC(M,1)=I	TRIA148
	NJUNC(M,2)=J	TRIA149
	SUB=T(3)+T(4)-T(2)	TRIA150
	G=SQRT(T(2))/2.	TRIA151
	LEN(M)=G	TRIA152
	C=G/SQRT(4.*T(3)*T(4)-SUB**2)*SUB	TRIA153
	G=G/2.*C	TRIA154
	IF(ISWCH(6).EQ.2)GO TO 690	TRIA155
	AS(I)=AS(I)+G/2.	TRIA156
	AS(J)=AS(J)+G/2.	TRIA157
690	CONTINUE	TRIA158
	IF(C.LE.0.) WRITE(N6,102) M,C	
	B(M)=B(M)+C	TRIA160
	R(M)=(DEP(I)+DEP(J))/2.	TRIA161
	AVGH(M)=(H(I)+H(J))/2.	TRIA162
	A(M)=B(M)*R(M)	TRIA163
	IF(ISWCH(4).EQ.0)GO TO 700	TRIA164
	IF(NN.EQ.3)AK(M)=EN(3)	TRIA165
	IF(NN.EQ.4)AK(M)=EN(4)	TRIA166
	GO TO 710	TRIA167
700	AK(M)=(COF(I)+COF(J))/2.	TRIA168
710	V(M)=0.	TRIA169
750	CONTINUE	TRIA170
	RETURN	TRIA171
	END	TRIA172
	ENDCOSY/	
	\$FORTRAN,I=54,L,M,X=40	
	\$REWIND,54	
	\$CROSSREF	
	\$INPUT=54	
	\$END	

ILLUSTRATIVE INPUT DATA SET FOR SWFLOW

0.10 0.05
0.35 0.05 0.05
0.05
0.19
0.85

0.10 0.05

0.05

0.02
0.03

4.2
31 6 3
0

0.45 0.08 0.05
0.13 0.08

0.03
0.18 0.05
0.02 0.02 0.04 0.02

0.01

0.05

5.0
30 6 3
2
18

0	0	0	0	265	491	484	479
479	480	483	482	479	476	471	467
466	468	467	469	471	473	475	1570
2307	2206	2161	2073	2010	1951		

CA2750209
CA2750210
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CA2750330

10 177 177 177 677 617 7

0.05

CA2750401
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CA2750430

0.01

0.10 0.01 0.05

0.27

0.12

0.02 0.05

0.02

0.03 0.05 0.02

0.05

0.42 0.20 0.20

0.03 0.03

0.35

4.25

31 6 3

1

10

0	0	0	0	0	56	189	235
148	57	54	56	61	66	396	585
272	30	671	-27	7	0	33	7
7	7	7	4	343	6777	6777	

CA2750501
CA2750502
CA2750503
CA2750504
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CA2750510
CA2750511
CA2750512
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CA2750516
CA2750517
CA2750518
CA2750519

1.25 0.47 0.26 1.00

0.21 0.27 0.20

0.20 1.47 1.68 1.17 0.31 1.51

0.02 0.06 0.02 0.40 0.15

0.05 1.87 1.23

0.02 1.50 0.92

0.03 0.02 0.07

0.06 0.03 0.05

0.24 1.23 2.00 1.22 0.11

0.21 1.24 0.25 0.03 0.81

0.07 0.08 0.11

0.10 0.15 0.03 0.16

0.28 0.20 0.47 1.19

0.12 0.20

0.10

				1.99	
	0.05			0.02	
0.17	0.60	0.35			
1.12	1.08	0.05	0.02		
0.33	0.33	1.15	0.50		1.00
0.05	0.09	0.08	0.08	0.99	0.66
	0.05	0.01	0.01	0.45	2.11
				0.28	0.11
1.70	2.70	3.22	0.13		
0.02	0.25	0.90	0.02		
0.95	1.92	3.47	1.77	0.72	0.14
0.60	0.02		0.02	0.32	0.05
0.30		0.32	0.20	0.19	
			0.05		

0.50	0.27				
	0.86	0.20	0.05	0.28	
	0.20		0.03	0.03	1.66
				0.23	0.05

0.03

0.13			0.05	0.01	
0.02	0.05		0.05		
				0.15	
	0.05		0.19	0.25	
			0.88	0.01	0.27
	0.28	0.33	2.65	0.29	0.35
0.35	2.63	0.04	0.10		0.20
	1.43	0.08	0.02		0.03
0.13	1.68	0.43	1.27		
	0.05	2.72	1.38	0.10	0.15
	0.05	0.06	0.18	1.37	0.20
		0.01	0.05	0.26	0.22
	0.05	0.03	0.10		
				0.20	
	0.15	0.50	0.02		
	2.90				

31

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17

659	656	659	679	686	659	662	696
810	926	883	863	851	816	803	790
776	767	758	748	729	431	0	0
0	0	0	0	0	0	0	

18

636	636	639	651	667	664	675	686
799	895	871	846	825	803	776	763
748	734	729	714	689	542	329	349
359	367	368	372	375	378	384	

14

636	634	639	648	662	664	675	680
784	859	851	838	821	799	767	753
739	729	719	699	679	499	324	344
354	356	363	368	371	372	375	

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6777	6777	6777	6777	0-2	6777	137	-57
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CA2750520
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[illegible]

2.74		0.22	0.20
		0.08	0.08
		0.05	
	0.18	2.40	
0.23		2.61	
	2.07	0.03	
0.80			
		0.04	
	0.13	0.37	0.32
1.33	1.21	0.14	0.48
ENDQUANTITY			

CA2750816
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 CA2750830
 CA2750831

ILLUSTRATIVE INPUT DATA SET FOR INDATA

\$JOB,8012-304,LIN,5
 \$EQUIP,20=MT
 \$RONL,854/6201
 \$FET,LIN,INDATA,960
 \$OPEN,40
 \$LOAD,40,M
 \$RUN

ILLUSTRATIVE INPUT DATA SETS FOR INDATA

HYDROLOGIC ANALYSIS OF WATER CONDS. AREA 2A ANALYSIS OF OCT. 1969 CONDITION

1 720	4 1	1 2	1 70								
1 24 3600	0 30	81 8	2 0	0 0	30 0	1 1					
9 10	11 12	13 14	15 16	17 18	19 20	21 22	23 24				
17 18	19 20	21 22	23 24								
25 26	27 28	29 30									
4 30	1 4	1 30	2 3	3 30	2 30	3 4	4 7				
3 7	5 7	4 5	7 8	3 8	2 8	8 28	2 28				
9 28	2 9	28 29	9 29	10 11	11 29	10 29	11 28				
10 12	11 12	12 13	11 13	13 24	11 24	24 25	11 25				
8 25	8 11	25 27	8 27	7 27	16 27	7 16	6 16				
6 7	5 6	15 16	15 27	21 26	16 21	16 26	21 22				
22 26	15 22	15 26	22 23	15 23	23 25	15 25	23 24				
17 21	17 22	17 18	18 22	18 23	14 18	14 23	14 24				
14 19	19 24	13 19	19 20	13 20	12 20	1 30	2 30				
2 9	9 29	10 29	2 3	1 4	4 5	5 6	17 18				
14 18											
1 30	2 10	8 6	18 16								

235.0	7.5	1.58								
113.75	40.017	0 5.	-8.20							29.57 1.16
213.78	104.410	0 5.	-9.50							25.8723.23
313.90	220.190	0	-11.00							35.9021.12
413.80	113.610	0 15.	-11.00							42.2415.05
513.80	133.080	0 15.	-11.00							58.0816.79
613.80	120.640	0 20.	-10.60							79.2019.01
714.00	299.718	0 0	-12.20							58.0831.68
814.01	330.550	0 0	-11.20							40.6641.98
913.80	88.186	0 10.	-12.00							16.3736.43
1013.90	157.970	0 0	-12.80							0.0059.14
1114.00	505.840	0 0	-12.50							26.4058.08
1214.10	203.960	0 0	-13.20							16.9086.38
1314.10	345.700	0 0	-13.00							31.6883.69
1414.05	97.380	20. 0	-12.50							54.9173.39
1514.05	106.036	0 0	-11.70							63.3655.97
1614.01	146.610	0 20.	-11.60							78.9940.39
1714.05	45.440	10. 0	-12.00							78.7865.47
1814.05	41.656	10. 0	-11.80							67.5869.17
1914.10	140.660	5. 0	-13.00							42.7791.34
2014.10	95.757	0 0	-13.50							30.62109.3
2114.02	54.640	0 20.	-11.80							78.9453.33
2214.05	77.900	0 0	-12.20							70.7560.19
2314.05	90.350	0 0	-11.80							58.8764.94
2414.05	260.220	0 0	-12.80							45.6768.64
2514.03	256.980	0 0	-11.60							49.1056.76
2614.03	122.265	0 0	-12.40							70.7549.10
2714.01	185.560	0 0	-12.00							59.9343.82
2814.00	233.710	0 0	-12.00							26.4043.03
29 13.80	161.759	0 5.	-12.50							84.4847.26
30 13.80	74.920	0 10.	-8.40							27.7212.14

77777

1 1	30 4
2 30	2 3

3	30	3	4
4	3	7	4
5	4	7	5
6	3	8	7
7	3	2	8
8	2	28	8
9	2	9	28
10	9	29	28
11	29	10	11
12	29	11	28
13	11	10	12
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15	11	13	24
16	11	24	25
17	8	11	25
18	8	28	11
19	8	25	27
20	8	27	7
21	7	27	16
22	7	16	6
23	5	7	6
24	27	15	16
25	16	26	21
26	26	22	21
27	26	15	22
28	15	23	22
29	15	25	23
30	15	27	25
31	25	24	23
32	22	17	21
33	22	18	17
34	22	23	18
35	23	14	18
36	23	24	14
37	24	19	14
38	24	13	19
39	13	20	19
40	13	12	20
41	1	30	0
42	30	2	0
43	2	9	0
44	9	29	0
45	29	10	0
46	2	3	0
47	1	4	0
48	4	5	0
49	5	6	0
50	18	17	0
51	14	18	0

10500.0	80.00	8.50	0.025
10600.0	80.00	8.50	0.025
15800.0	60.00	7.00	0.028
13700.0	60.00	5.00	0.025
14800.0	55.00	5.00	0.025
10500.0	85.00	-7.00	0.050
18500.0	70.00	8.50	0.025
15800.0	65.00	6.50	0.030
21100.0	65.00	6.00	0.030
12600.0	35.00	0.00	0.030
12600.0	35.00	0.00	0.030

99999

A SAMPLE OUTPUT

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[illegible]

SAMPLE OUTPUT OF SWFLOW PROGRAM